PROJECT MANUAL

For Construction of:

PROJECT NO.: 20130026

ALPHARETTA CONFERENCE CENTER AND HOTEL AT AVALON

ALPHARETTA, GEORGIA

Prepared for:

STORMONT HOSPITALITY GROUP, LLC ATLANTA, GEORGIA

NORTH AMERICAN PROPERTIES ATLANTA, GEORGIA

ISSUED FOR DESIGN DEVELOPMENT

DATE: FEBRUARY 12, 2016

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Prepared For:

STORMONT HOSPITALITY GROUP, LLC 3350 RIVERWOOD PARKWAY, SUITE 1590 ATLANTA, GEORGIA 30339 (770) 693-0586

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ISSUED FOR DESIGN DEVELOPMENT

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To be Issued

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MISCELLANEOUS

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012600	Contract Modification Procedures
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Cast-In-Place Concrete	
Unbonded Post-Tensioned Concrete	
Precast Architectural Concrete	To be Issued
Concrete Topping	To be Issued
	Unbonded Post-Tensioned Concrete Precast Architectural Concrete

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064214	Stile and Rail Wood Paneling
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071413	Hot Fluid-Applied Rubberized Asphalt Waterproofing
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071800	Traffic Coatings
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074213.13	Formed Metal Wall Panels
074213.23	Metal Composite Material Wall Panels
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076200	Sheet Metal Flashing and Trim
077100	Roof Specialties
077129	Manufactured Roof Expansion Joints
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078100	Applied Fireproofing
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081416	Flush Wood Doors		
081433	Stile and Rail Wood Doors	To be Issued	
083113	Access Doors and Frames		
083323	Overhead Coiling Doors		
084113	Aluminum-Framed Entrances and Storefronts		
084413	Glazed Aluminum Curtain Walls		
085123.13	Hot-Rolled Steel Windows		
087100	Door Hardware		
088000	Glazing		
088300	Mirrors	To be Issued	
089119	Fixed Louvers		
Division 00 Finishes			
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092116.23	Gypsum Board Shaft Wall Assemblies		

092116.23	Gypsum Board Shaft Wall Assemblies	
092216	Non-Structural Metal Framing	
092900	Gypsum Board	
093013	Ceramic Tiling	
093033	Stone Tiling	To be Issued
095113	Acoustical Panel Ceilings	
096513	Resilient Base and Accessories	
096340	Stone Flooring	To be Issued
096519	Resilient Tile Flooring	
096816	Sheet Carpeting	To be Issued
097200	Wall Coverings	
097513	Stone Wall Facing	To be Issued
099113	Exterior Painting	
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099600	High-Performance Coatings	To be Issued
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099653	Elastomeric Coatings	To be Issued

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101419 101423	Dimensional Letter Signage Panel Signage	To be Issued
102113.16	Plastic-Laminate-Clad Toilet Compartments	
102239.13	Folding Glass-Panel Partitions	
102600	Wall and Door Protection	
102800	Toilet, Bath, and Laundry Accessories	
102819	Shower Doors	To be Issued
103100	Manufactured Fireplaces	
104413	Fire Protection Cabinets	To be Issued
104416	Fire Extinguishers	
105113	Metal Lockers	

Division 11 - Equipment

111313	Loading Dock Bumpers
111319	Stationary Loading Dock Equipment
112429	Tie-Back and Life-Line Anchors
114000	Food Service Equiipment
115213	Projection Screens
118226	Facility Waste Compactors

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122113	Louver Blinds
122413	Roller Window Shades
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123623.13	Plastic-Laminate-Clad Countertops
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124813	Entrance Floor Mats and Frames

Division 13 - Special Construction

131100 Swimming Pools

Division 14 - Conveying Equipment

142100	Electric Traction Elevators	To be Issued
142400	Hydraulic Elevators	To be Issued
149133	Laundry and Linen Chutes	

Division 15 to Division 19 (Not Applicable)

Division 20 (Not Applicable)

Division 21 - Fire Suppression

215250 Fire Suppression Systems

Division 22 - Plumbing

221113	Facility Water Distribution Piping
221313	Facility Sanitary Sewers
222300	Packaged Booster Pumping System
224000	Plumbing Systems
224240	Water Heaters & Accessories
224500	Plumbing Fixtures & Trim

Division 23 - Heating, Ventilating, and Air-Conditioning (HVAC)

230002	Building Mechanical Systems
230100	Mechanical General
230430	HVAC Test and Balance
230600	HVAC Piping Systems
231410	Centrifugal Pumps
231720	Variable Frequency Drives
231810	HVAC Piping Insulation
231900	Electric Heat Tracing
232500	Water Treatment
236720	Packaged Air-Cooled Rotary Chillers
236740	Frictionless Centrifugal Chillers
236800	Cooling Towers
237400	Terminal Units

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237600	Air Handling Units
237650	Fan Coil Units
237660	Modular High-Rise Fan Coil Units
237730	Split Systems
237790	Energy Recovery Units
238000	Air Distribution Devices
238200	Fans
238400	Ductwork
239000	Automatic Controls and Energy Management System

Division 24 (Not Applicable)

Division 25 - Integrated Automation (Not Applicable)

Division 26 - Electrical

260002	Building Electrical Systems
260100	Electrical General
261000	Electrical Basic Materials & Methods
262000	Service & Distribution
263000	Lighting
265101	Architectural Lighting Fixtures
265102	Building Exterior Lighting Cutsheets
265103	Building Interior Lighting Cutsheets
265104	Landscape Lighting Cutsheets
265105	Architectural Lighting Controls
266010	Lightning Protection
266100	Emergency System
266500	Surge Protective Device
269200	Motor Controls & Wiring

Division 27 - Communications (Not Applicable)

Division 28 - Electronic Safety and Security

287210 Life Safety System

Division 29 (Not Applicable)

Division 30 (Not Applicable)

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311000	Site Clearing
312000	Earth Moving
312319	Dewatering
313180	Rock Excavation
245000	Evenuetion Common

315000 Excavation Support and Protection

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321216	Asphalt Paving
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321313	Concrete Paving
321313.26	Colored Concrete Paving
321400	Unit Paving
321723	Pavement Marking and Removal
328400	Planting Irrigation
329100	Finish Grading
329200	Landscaping
329219.16	Hydraulic Seeding
329223	Sodding

Division 33 - Utilities

330500 Common Work Results for Utilities 334100 Storm Utility Drainage Piping

Division 34 to Division 49 (Not Applicable)

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RESPONSIBILITY MATRIX

RESPONSIBILITY CODES:

Budget Categories

GC	Basic construction, prepared by the General Contractor
FFF	Furniture fixtures and equipment

Team Members

AC	. Acoustical Consultant
AE	
ART	
CIV	. Civil Engineer
EE	
GC	
GD	
ID	
KC	
LA	•
LC	
LD	
LS	
ME	
O	
OP	
OS	
OS&E	
PUR	
SE	
SL	
SPECIAL	Pool Designer (Design/Build) hired by GC
TC	
WF	.Water Feature Consultant

ITEM		BUDGET	DESIGN	CONTRACT DOCUMENT	PURCHASE	INSTALL	
A. (GUESTR	OOMS (ALL VARIETIES)					
1	a. Co b. In	none System: onduit, Pull Wire, Boxes stall Wire or Teflon Cable in ou of Conduit if Code Permits	GC GC	AE/ID/TC TC	AE/TC TC	GC GC	GC GC
2	a. Co Bo	sion System - Cable: onduit, Pull Wire, Sleeves, and oxes	GC	AE/ID	AE/ID	GC	GC
	c. Co	stall Wire or Teflon Cable over Plate onnect Television to Cover Plate	GC GC FFE	GC AE GC	GC AE GC	GC GC	GC GC
3	3. Mecha a. Co	nical: omplete System	GC	AE	AE	GC	GC

	ITEM	BUDGET	DESIGN	CONTRACT DOCUMENT	PURCHASE	INSTALL
4.	Electrical: a. Complete System b. Special Decorative Lighting c. Suspended Chandeliers d. Wall Sconces	GC FFE FFE FFE	AE ID/LD ID/LD ID/LD	AE AE/ID AE/ID AE/ID	GC O O	GC GC GC
5.	Sprinkler: a. Complete System	GC	GC	GC	GC	GC
6.	Smoke Detectors/Voice Data/Alarm:	00	45	45	22	00
	a. Complete System	GC	AE	AE	GC	GC
7.	Ceilings: a. Standard b. Entry Foyer	GC GC	AE/ID AE/ID	AE/ID AE/ID	GC GC	GC GC
8.	Wall Covering: a. Vinyl/Fabric (OS) b. Ceramic (OS) c. Vinyl/Fabric (Special)	FFE GC FFE	ID AE ID	ID AE ID	O GC O	GC GC GC
9.	Flooring: a. Carpet/Pad (OS) b. Carpet/Pad (Special) c. Ceramic d. Base	FFE FFE GC GC	ID ID AE AE/ID	ID ID AE AE/ID	O O GC GC	GC GC GC
10.	Doors/Frames/Hardware: a. Card-Operated Hardware	GC	AE/ID	AE	GC	GC
11.	Blocking: a. For Furniture, Toilet Accessories, Millwork, Artwork, Specialties, and Special Construction	GC	AE	AE	GC	GC
12.	Furniture: a. Standard Room (OS) b. Special Room	FFE FFE	ID ID	ID ID	0	0
13.	Special Construction (OS): a. Vanity Tops b. Mirrors c. Closet, Self, Rod, Hangers/Hooks d. Toilet Accessories e. Toilet Fixtures f. Shower Rod g. Hair Dryer (Allowance of \$25 Per Unit to Purchase)	GC GC GC GC GC GC	AE AE AE AE AE AE	AE AE AE AE AE AE	GC GC GC GC GC	GC GC GC GC GC
14.	Artwork: a. Artwork (OS) b. Artwork (Special) c. Blocking (Special)	FFE FFE GC	ID ID AE	ID ID AE	O O GC	O O GC
15.	Graphics/Signage: (N.I.C.) a. Coordinate for Electrical/Blocking, if required	GC	AE/GD	AE	GC	GC

		ITEM	BUDGET	DESIGN	CONTRACT DOCUMENT	PURCHASE	INSTALL
	16.	Draperies/Track:			_	_	
		a. Draperies/Track (OS)	FFE	ID	ID	0	0
		b. Draperies/Track (Special)	FFE	ID	ID ID	O GC	0
		c. Blocking/Nailers	GC	ID	ID	GC	GC
	17.	Refrigerators:	FFE	KC	AE/KC	0	0
	18.	Special Suites; Rooms, Corridors, and Meeting Suites:					
		a. Millwork	GC	AE/ID	AE/ID	GC	GC
		b. Vanity and Bar Tops	GC	ID	ID	GC	GC
		c. Plumbing Fixtures	GC	AE/ID	AE	GC	GC
		d. Electric, Standard	GC	AE	AE	GC	GC
		e. Special Fixtures (Lighting)	FFE	ID	ID	0	GC
		f. Special Floor Treatment	GC	ID	AE/ID	GC	GC
		g. Bar/Pantry Equipment	GC	KC	AE/KC	GC	GC
		h. Rough-In/Final Hook-Up	GC	AE/KC	AE/KC	GC	GC
		i. Doors and Hardware j. Card-Operated Hardware	GC GC	AE/ID AE/ID	AE AE	GC GC	GC GC
			00	ALIID	AL	00	00
В.		JESTROOM CORRIDORS AND LEVATOR LOBBY					
	1.	Electrical:					
		a. Lighting (OS)	GC	AE/LD	AE	GC	GC
		b. Lighting (Scones)	FFE	ID/LD	AE/ID	0	GC
		c. Lighting (Plug-In)	FFE	ID/LD	ID	0	0
		d. Vending Area	GC	KC	AE/KC	GC	GC
		e. Rough-In/Hook-Up of Equipment	GC	AE/KC	AE/KC	GC	GC
		f. Complete System	GC	AE	AE	GC	GC
	2.	Millwork:	GC	AE/ID	AE/ID	GC	GC
	3.	Vending Equipment:				_	
		a. Equipment	FFE	KC	AE/KC	0	N/A
		b. Install Equipment	N/A	AE/KC	AE/KC	N/A	0
		c. Hook-Up Equipment	GC	AE/KC	AE/KC	GC	GC
	4.	Signage: (N.I.C.) a. Room Number Directional (OS)	GC	AE/CD	A E	0	GC
		a. Room Number Directional (OS) b. Miscellaneous (Special)	FFE	AE/GD GD/ID	AE ID	0 0	GC
		· · ·			i.b	_	
	5.	Artwork:	FFE	ID	ID	0	GC
	6.	Art Urns:		45/00			
		a. Fixed	FFE	AE/GD	AE	0	GC
		b. Moveable	FFE	ID	ID	0	0
	7.	Furniture and Freestanding Items (Special):	FFE	ID	ID	0	GC
	8.	Fire and Life Safety:	00	4.5	. –	00	00
		a. Fire Extinguisher	GC	AE	AE AE	GC	GC
		b. Fire Cabinet and Hose	GC	AE	AE	GC	GC
		c. Fire Alarm and Annunciator	GC	AE (CC)	AE (CC)	GC	GC
		d. Sprinklers	GC	(GC)	(GC)	GC	GC
		e. Smoke Detectors	GC	AE	AE	GC	GC
		f. Voice Communication	GC	AE	AE	GC	GC

FEBRUARY 12, 2016 HOTEL AT AVALON - 20130026 4 RESPONSIBILITY MATRIX

HC	TEL	_ AT AVALON - 20130026			RESP	ONSIBILITY M	ATRIX
		ITEM	BUDGET	DESIGN	CONTRACT DOCUMENT	PURCHASE	INSTALL
	9.	Blocking:	GC	AE	AE	GC	GC
	10.	Mirrors (Special Design):	FFE	ID	ID	O	GC
	11.	Planters Interior:					
		a. Fixed (Special Design)	GC	ID	AE/ID	GC	GC
		b. Moveable	FFE	ID	ID	0	GC
	12.	Plants and Materials:	FFE	LA	LA	O	0
	13.	Ceiling:	GC	AE/ID	AE/ID	GC	GC
	14.	Wall Covering:				_	
		a. Vinyl/Fabric (OS)	FFE	ID ID	ID AE/ID	0	GC
		b. Vinyl/Fabric (Special) c. Paint	FFE GC	AE/ID	AE/ID AE/ID	O GC	GC GC
		c. rant	00	ALID	ALIID	00	00
	15.	Floors:					
		a. Carpet/Pad (OS)	FFE	ID	ID	0	GC
		b. Carpet/Pad (Special)	FFE	ID	ID	0	GC
		c. Hard Finishes (Special) d. Wall Base	GC GC	ID ID	AE/ID AE/ID	GC GC	GC GC
		u. Wali base	90	טו	ALID	GC	90
	16.	Doors/Frames/Hardware:					
		a. Card-Operated Hardware	GC	AE	AE	GC	GC
		b. Exit Hardware	GC	AE	AE	GC	GC
		c. Millwork d. Finishes	GC GC	AE/ID AE/ID	AE/ID AE/ID	GC GC	GC GC
		d. Finishes	GC	AE/ID	AE/IU	GC	GC
	17.	Elevator/Public Areas:					
		a. Cab Complete	GC	AE/ID	AE/ID	GC	GC
		b. Frames/Doors/Guestroom Floors	GC	AE/ID	AE/ID	GC	GC
		c. Signage (Built-In)	GC	ID	AE/ID	GC	GC
		d. Lobby Devices Guest Floors e. Doors/Public Floors	GC GC	AE AE/ID	AE AE/ID	GC GC	GC GC
		e. Doors/Public Floors f. Signage Applied	GC	ID	AE/ID ID	GC	GC
	18	Window Treatments:	FFE	ID	ID	0	0
		Wildow Froditionio.		15		· ·	· ·
C.	LIN	NEN ROOM/SERVICE ELEVATOR	LOBBY				
	1.	Shelving:	0005	4.5	A.F.	•	•
		a. Movable Shelvingb. Built-In Shelving	OS&E GC	AE AE	AE AE	O GC	O GC
		b. Built-in Shelving	GC	AE	AL	GC	GC
	2.	Sink:	GC	AE	AE	GC	GC
	3.	Linen Chute:	GC	AE	AE	GC	GC
	4.	Finishes:	GC	AE	AE	GC	GC
D.	ST	AIRWELLS					
	1.	Finishes/Rail/Stair:	GC	AE	AE	GC	GC
E.	PC	OOL/SPA AREAS					
	1.	Pool/Spa and Equipment:	GC	GC(D/B)	GC(D/B)	GC	GC

		ITEM	BUDGET	DESIGN	CONTRACT DOCUMENT	PURCHASE	INSTALL
	2.	Special Lighting:	GC	GC(D/B)/ LD(ag)	GC(D/B)	GC	GC
	3.	Special Plumbing:	GC	GC(D/B)	GC(D/B)	GC	GC
	4.	Lockers/Benches:	GC	AE/ID	AE/ID	GC	GC
	5.	Finishes:	GC	AE/ID	AE	GC	GC
	6.	Doors/Frames/Hardware:	GC	AE	AE	GC	GC
	7.	Fire Hose Cabinet:	GC	AE	AE	GC	GC
	8.	Fire Extinguishers:	GC	AE	AE	GC	GC
	9.	Pool Furniture:	FFE	ID	ID	Ο	GC
F.	Pι	JBLIC AREA					
	1.	Lobby and Entries: a. Hard Flooring b. Carpet or Rugs	GC FFE	ID ID	AE/ID ID	GC O	GC GC
	2.	Ceilings:	GC	AE/ID	AE/ID	GC	GC
	3.	Walls: a. Drywall b. Wallcoverings c. Storefronts, Etc. d. Special Finishes e. Millwork	GC FFE GC GC GC	AE/ID ID AE/ID ID ID	AE/ID ID AE AE/ID ID	GC O GC GC	GC GC GC GC
	4.	Lighting: a. Decorative and Plug-In b. General c. Audio Visual d. Dimmers	FFE GC GC GC	ID/LD AE/ID/LD SL AE/LD	ID AE/ID AE AE	O GC GC	GC GC GC
	5.	Fire Suppression Per Code: a. Sprinklers b. Fire Alarm c. Cabinet, Hose, and Extinguishers d. Smoke Detectors e. Voice Communication	GC GC GC GC	GC AE AE AE AE	GC AE AE AE AE	GC GC GC GC	GC GC GC GC
	6.	Mechanical: a. Complete System	GC	AE	AE	GC	GC
	7.	Specialties: a. Moveable Planters, Draperies b. Signage	FFE GC	GD/ID GD/ID	ID GD	0 0	O GC
	8.	Furniture: a. Moveable and Fixed	FFE	ID	ID	0	GC
	9.	Artwork and Artifacts:	FFE	ID	ID	o	GC

		ITEM	BUDGET	DESIGN	CONTRACT DOCUMENT	PURCHASE	INSTALL
	10.	Millwork:	GC	ID	ID	GC	GC
G.	RE	STAURANT, BAR, LOUNGE, GIF	T SHOP				
	1.	Flooring: a. Hard Flooring	GC	ID	AE	GC	GC
		b. Carpet or Rugs	FFE	ID	ID	0	GC
	2.	Ceilings:	GC	ID	AE/ID	GC	GC
	3.	Walls:					
		a. Drywallb. Wallcoverings	GC FFE	AE/ID ID	AE/ID ID	GC O	GC GC
		b. Wallcoverings	FFE	טו	ID	O	GC
	4.	Doors/Frames/Hardware:	GC	AE/ID	AE	GC	GC
	5.	Lighting: a. Fixed/Built-In Fixtures	GC	AE/ID/LD	AE	GC	GC
		b. Suspended or Wall-Mounted	FFE	ID/LD	AE/ID	0	GC
		Fixtures					
		c. Sound System	FFE	SL	AE/SL	GC	GC
		d. Special Lounge Lighting	FFE	ID/SL	AE/ID/SL	0	GC
	6.	Fire and Life Safety per code:	00		00		
		a. Sprinklers b. Fire Alarm	GC GC	GC AE	GC AE	GC GC	GC GC
		c. Cabinet, Hose, and Extinguishers	GC	AE	AE	GC	GC
		d. Smoke Detectors	GC	AE	AE	GC	GC
		e. Voice Communication	GC	AE	AE	GC	GC
	7.	Mechanical:					
		a. Complete System	GC	AE	AE	GC	GC
	8.	Specialties, Furniture, Artwork, and Equipment:	FFE	ID	ID	0	GC
	9.	Signage:	GC	GD	GD	GC	GC
Н.	ME	EETING ROOMS					
	1.	Flooring:					
		a. Carpet and Pad	FFE	ID	ID	0	GC
		b. All Other Systems	GC	ID	ID	GC	GC
	2.	Ceilings:	GC	AE/ID	AE/ID	GC	GC
	3.	Walls:					
		a. Drywallb. Wallcoverings	GC FFE	AE/ID ID	AE/ID AE/ID	GC O	GC GC
		b. Wallcoverings c. Fabric-Covered Panels	FFE	ID ID	ID	GC	GC
		d. Millwork	GC	ID	ID	GC	GC
		e. Millwork Blocking	GC	AE/ID	AE/ID	GC	GC
	4.	Doors/Frames/Hardware:	GC	AE/ID	AE/ID	GC	GC
	5.	Lighting:		. =	. –		
		a. Built-In	GC	AE/ID/LD	AE	GC	GC

		ITEM	BUDGET	DESIGN	CONTRACT DOCUMENT	PURCHASE	INSTALL
		b. Suspended or Wall Mounted (Including Chandelier)	FFE	ID/LD	AE/ID	0	GC
		c. Chandelier Assembly (Including Crystal)	FFE	ID/LD	AE/ID	0	GC
	6.	Fire and Life Safety Per Code:					
		a. Sprinklers	GC	GC	GC	GC	GC
		b. Fire Alarm	GC	ΑE	AE	GC	GC
		c. Cabinet, Hose, and Extinguishers	GC	AE	AE	GC	GC
		d. Smoke Detectors	GC	AE	AE	GC	GC
		e. Voice Communication	GC	AE	AE	GC	GC
	7.	Mechanical:					
		a. Complete System	GC	AE	AE	GC	GC
	8.	Specialties/Furniture/Artwork/ Graphics/ Draperies/Planters/ Kiosks:	FFE	ID	ID	0	0
	9.	Moveable Partitions:					
	•	a. Wall System/Pocket Doors	GC	ΑE	ΑE	GC	GC
		b. Wall Finish (Special)	GC	ID	AE/ID	GC	GC
	10.	Graphics/Signage: (N.I.C.)	GC	GD	GD	GC	GC
I.	ΑC	MINISTRATIVE AND MISCELLAN	NEOUS OF	FICES			
	1.	Flooring:					
		a. Carpet/Pad	FFE	ID	ID	0	GC
		b. Sealed Concrete	GC	AE	AE	GC	GC
	2.	Ceilings:					
		a. Acoustical Lay-In	GC	AE/ID	AE/ID	GC	GC
	3.	Walls:					
		a. Paint	GC	ID	AE/ID	GC	GC
		b. Wallcoverings	FFE	ID	AE/ID	0	GC
	4.	Doors/Frames/Hardware:	GC	AE	AE	GC	GC
	5.	Lighting:					
		a. Fixed/Built-In Fixtures	GC	AE/ID	AE/ID	GC	GC
		b. Decorative/Wall-Mounted Fixtures	FFE	ID	AE/ID	0	GC
	6.	Fire Suppression Per Code:					
		a. Complete System	GC	GC	GC	GC	GC
	7.	Mechanical:					
	••	a. Complete System	GC	AE	AE	GC	GC
	8.	Specialties, Furniture, and Artwork:	GC	AE	AE	GC	GC
	0.	opecialies, i ullillule, allu Altwork:	GC	AL	AE	GC	GC
	9.	Signage:	GC	GD	GD	GC	GC

		ITEM	BUDGET	DESIGN	CONTRACT DOCUMENT	PURCHASE	INSTALL
J.	PU	IBLIC AND EMPLOYEE TOILETS					
	1.	Flooring: a. Public b. Employee	GC GC	AE/ID AE	AE AE	GC GC	GC GC
	2.	Ceilings: a. Public b. Employee	GC GC	AE/ID AE	AE AE	GC GC	GC GC
	3.	Walls: a. Public b. Employee	GC GC	AE/ID AE	AE AE	GC GC	GC GC
	4.	Doors/Frames/Hardware:	GC	AE	AE	GC	GC
	5.	Lighting: a. Public b. Employee	GC GC	AE/ID/LD AE	AE AE	GC GC	GC GC
	6.	Fire Suppression Per Code: a. Complete System	GC	(GC)	(GC)	GC	GC
	7.	Mechanical: a. Complete System	GC	AE	AE	GC	GC
	8.	Specialties, Toilet Partitions, and Accessories: a. Public b. Employee	GC GC	AE/ID AE	AE AE	GC GC	GC GC
	9.	Lockers: a. Lockers	GC	AE	AE	GC	GC
K	. F	ITNESS					
	1.	Finishes:	еÇ	^ ₹E	A E E	e ^C C	e ^C C
	2.	Equipment:	дс	^ ₹E	ĀĘE	вс	дc
L.		TCHEN, LAUNDRY, DUSEKEEPING, RECEIVING					
	1.	Flooring:	GC	AE	AE	GC	GC
	2.	Ceilings:	GC	AE	AE	GC	GC
	3.	Walls:	GC	AE	AE	GC	GC
	4.	Doors/Frames/Hardware:	GC	AE	AE	GC	GC
	5.	Lighting:	GC	AE	AE	GC	GC
	6.	Fire Suppression and Mechanical: a. General Systems b. Hood and Extinguisher System	GC FFE	GC KC	GC AE/KC	GC GC	GC GC

		ITEM	BUDGET	DESIGN	CONTRACT DOCUMENT	PURCHASE	INSTALL
		c. Rough-In for Equipment	GC	AE/KC/L C	AE/KC/LC	GC	GC
	7.	Kitchen Equipment:	GC	кс	КС	GC	GC
	8.	Laundry Equipment:	GC	LC	LC	GC	GC
М.	MI	SCELLANEOUS BACK-OF-HOUS	SE AREAS				
	1.	Flooring:	GC	AE	AE	GC	GC
	2.	Ceilings:	GC	AE	AE	GC	GC
	3.	Walls:	GC	AE	AE	GC	GC
	4.	Doors/Frames/Hardware:	GC	AE	AE	GC	GC
	5.	Lighting:	GC	AE	AE	GC	GC
	6.	Fire Suppression and Mechanical:					
			00	00	00	00	00
		a. General Systems	GC	GC	GC	GC	GC
		b. Hood and Extinguisher Systemc. Rough-In for Equipment	GC GC	KC AE	AE/KC AE	O GC	GC GC
N.	MI	SCELLANEOUS		/.=	7.=		
	1.	Sound:					
		a. Equipment (Speakers, Amps, Controls, Mikes, Hook-Up)	GC	SL	AE/SL	GC	GC
		b. Conduit and Power	GC	ΑE	AE	GC	GC
		c. Furnish Wire	GC	SL	SL	GC	GC
		d. Back Boxes, Junction Boxes	GC	ΑE	ΑE	GC	GC
		e. Cable Installation	GC	SL	SL	GC	GC
	2.	P.O.S.:					
		a. Units	OS&E	KC	AE/KC	0	0
		b. Conduit and Pull Wire	GC	ΑE	ΑE	GC	GC
		c. Power Wire (Clean to Ground)	GC	ΑE	ΑE	GC	GC
		d. Intercommunicating Wire	GC	ΑE	AE	GC	GC
		e. Wire Pulling	GC	AE	AE	GC	GC
	3.	P.M.S.:					
		a. Units	OS&E	0	AE/O	0	0
		b. Electric and HVAC	GC	ΑE	AE	GC	GC
		c. Power Wire (Clean to Ground)	GC	ΑE	AE	GC	GC
		d. Intercommunicating Wire	GC	ΑE	AE	GC	GC
		e. Conduit	GC	ΑE	AE	GC	GC
		f. Electric Hook-Up System	GC	ΑE	AE	GC	GC
		g. Fire Protection System	GC	AE	AE	GC	GC
	4.	Television System - Cable:		. –			
		a. Conduit, Pull Wire, Sleeves, and Boxes	GC	AE	AE	GC	GC
		b. Install Wire or Teflon Cable	GC	TC	0	GC	GC
		c. Cover Plate	GC	AE	AE	GC	GC

FEBRUARY 12, 2016 HOTEL AT AVALON - 20130026

RESPONSIBILITY MATRIX

		ITEM	BUDGET	DESIGN	CONTRACT DOCUMENT	PURCHASE	INSTALL
	5.	Telephone: a. Conduit, Sleeves, Boxes, and Pull Wire (Based on a Private System)	GC	тс	AE/TC	GC	GC
		 b. Install Wire c. Devices d. Jacks e. Public Phone Enclosures f. System Equipment 	GC O GC FFE/GC O	TC AE/TC AE/TC AE/ID TC-PM (contract	TC AE/TC AE/TC AE/ID AE/TC	GC GC GC/O O	GC GC GC/O O
				with owner)			
Ο.	SI	TEWORK/LANDSCAPING					
	1.	Demolition/Excavation:	GC	AE/CIV	AE/CIV	GC	GC
	2.	Site Grading Plan:	GC	CIV	CIV	GC	GC
	3.	Site Lighting: a. Parking/Street Lighting	GC	LA/CIV/E E	CIV/EE	GC	GC
		b. Landscape Lighting	GC	LA/LD	LA/EE	GC	GC
		c. Building Lighting	GC	LD/AE/E E	LD/EE	GC	GC
	4.	Site Signage:					
		a. Hotel Identification (N.I.C.)	GC	GD	GD	GC	GC
		b. Sign Supportc. Description/Traffic and Striping	GC GC	LA LA/CIV	GC CIV	GC GC	GC GC
	5.	Site Paving:					
		a. Roadways/Curbs	GC	LA/AE	CIV	GC	GC
		b. Sidewalks	GC	LA/AE	CIV	GC	GC
		c. Special Paving c. Pedestrian Path	GC GC	LA LA/CIV	LA/CIV LA/CIV	GC GC	GC GC
	•		00	LAVOIV	LAVOIV	00	00
	6.	Site Utilities: a. Sewer/Storm and Site Drainage/Gas	GC	CIV/ME	CIV/ME	GC	GC
		b. Water Mains/Fire Hydrants	GC	CIV/ME	CIV/ME	GC	GC
		c. Electrical/Telephone/CATV	GC	CIV/EE	CIV/EE	GC	GC
		d. Transformer Vault	GC	CIV/EE	CIV/EE	GC	GC
		e. Site Electrical Outlets	GC	EE	CIV/EE	GC	GC
		f. Utility Connections	GC	CIV	CIV	GC	GC
		g. Site Hose Bibbs h. Emergency Generator	GC GC	LA AE/EE	LA/CIV EE	GC GC	GC GC
	_	•	00	ALILL	LL	00	00
	7.	Landscaping: a. Site Retaining Walls/Planters	GC	LA/CIV	LA/SE	GC	GC
		b. Irrigation/Drainage	GC	LA/CIV	LA/ME	GC	GC
		c. Planting	GC	LA/naturalis	LA	GC	GC
		d. Water Features/Fountains	GC	LA/WF/S E	LA/ME/EE/WF	GC	GC
		e. Site Furniture	GC	LA/ID	LA/ID	PUR	GC
		f. Movable Planters/Soil	GC	LA/ID	LA/ID	PUR	GC
		g. Tree Preservation	GC	LA/CIV	LA/CIV	GC	GC

8. Loading Dock Area: (Not Applicable)

FEBRUARY 12, 2016

O Cradinar			DOCUMENT		
9. Grading: a. Fine Grading	GC	LA/CIV	LA/CIV	GC	GC
P. EXTERIOR SIGNAGE					
1. Complete System:	0	AE/GD	AE/GD	GC	0
2. Blocking:	GC	AE/GD	AE/GD	GC	GC
3. Power:	GC	AE/GD	AE/GD	GC	GC

END OF RESPONSIBILITY MATRIX

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GEOTECHNICAL DATA

SECTION 003132

GEOTECHNICAL DATA

PART 1 - GENERAL

1.1 RELATED DOCUMENTS

A. Drawings and general provisions of the Contract, including General and Supplementary Conditions, Division 01 and other applicable Specification Sections, apply to this Section.

1.2 SUMMARY

- A. Scope: A geotechnical survey has been done for the site and is attached at the end of this Section.
- B. Geotechnical Report: The geotechnical report is the Report of Geotechnical Exploration; Avalon Hotel, Conference Center & Parking Deck, Alpharetta, Fulton County Georgia, prepared for Stormont Hospitality Group, LLC, Riverwood 100, 3350 Riverwood Parkway, Suite 1590, Atlanta, Georgia 30339, by Contour Engineering, LLC, 1955 Vaughn Road, Suite 101, Kennesaw, Georgia 30144; dated April 29, 2014.

1.1 SITE INVESTIGATION DATA

- A. The geotechnical report is not a part of the Contract Documents. The Contractor who obtains a copy of the report releases the Owner and the Architect from any responsibility or obligation as to its accuracy or completeness.
- B. Interpretations: The data in the geotechnical report regarding subsurface conditions and materials has been obtained by the Owner for the Architect's use in designing the Project. Neither the Owner nor the Architect guarantee the accuracy or completeness of the information in the report.
 - 1. The Contractor assumes responsibility for all excavation and earthwork for this Project and shall not rely on information obtained from the Architect or the Owner, directly or indirectly.
 - 2. The Contractor may make their own investigation of existing subsurface conditions.
 - 3. Neither the Owner or the Architect are or will be responsible or obligated for additional compensation for work performed under the Contract due to Contractor assumptions based on use of information in the report.
 - 4. The Contractor shall notify the Owner and the Architect in writing immediately upon discovery or identification of any discrepancy between site conditions and information in the geotechnical report.

PART 2 - PRODUCTS (Not Applicable)

PART 3 - EXECUTION (Not Applicable)

END OF SECTION 003132

SECTION 003132 - 2 GEOTECHNICAL DATA FEBRUARY 12, 2016

ALPHARETTA CONFERENCE CENTER AND HOTEL AT AVALON - 20130026

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Geotechnical Services • Materials Testing Services • Environmental Services

1955 Vaughn Road, Suite 101 Kennesaw, Georgia 30144 (770) 794-0266 www.contoureng.com

REPORT OF GEOTECHNICAL EXPLORATION Avalon Hotel, Conference Center & Parking Deck Alpharetta, Fulton County, Georgia Project No.: G14NAP02 April 29, 2014

Prepared For:

Stormont Hospitality Group, LLC Riverwood 100 3350 Riverwood Parkway, Suite 1590 Atlanta, Georgia 30339 Geotechnical Services • Materials Testing Services • Environmental Services

April 29, 2014

Stormont Hospitality Group, LLC

Riverwood 100 3350 Riverwood Parkway, Suite 1590 Atlanta, Georgia 30339

Attention: Mr. Jim Stormont

President

Reference: Report of Geotechnical Exploration

Avalon Hotel, Conference Center & Parking Deck

Alpharetta, Fulton County, Georgia Contour Project No.: G14NAP02

Dear Jim:

Contour Engineering, LLC has completed the geotechnical exploration for the project referenced above in general accordance with our Proposal No. G14NAP-103RII dated March 24, 2014.

We appreciate the opportunity to work with you on this project and look forward in assisting you with any future projects. Should you have any questions regarding this report or if we may be of further service, please contact our office.

Sincerely,

Contour Engineering, LLC

James E. Gough, P.E.

Geotechnical Services Manager

Jack M. Rebeiz, P.E.

President

Principal Engineer

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APPENDIX

Site Vicinity Map
Boring Location Plan
Test Pit Location Plan
Photographic Documentation
Subsurface Cross-Sections (3)
Boring Logs (31)
Test Pits Logs (14)
Soil Classification Chart
Retaining Wall Detail
Typical Benching Detail
Typical Backfill/Retaining Wall Drainage Detail



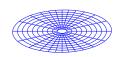
1.0 EXECUTIVE SUMMARY

This executive summary presents an overview of pertinent findings, conclusions, and recommendations. This overview should not be utilized in design or construction without reviewing the entire report.

- The project site lies within the eastern portion of the Avalon development located in Alpharetta, Georgia. Proposed for development is the construction of a Hotel and Conference Center with an approximate 97,500 square foot building footprint. The proposed hotel will be a fifteen-story tower with column loads ranging from approximately 300 to 1150 kips. The conference center will be a one-story structure with column loads ranging from approximately 25 to 175 kips. In addition to the Hotel and Conference Center, a four-story parking deck will be constructed. The proposed parking deck will have column loads ranging from approximately 275 kips to 750 kips and maximum wall loads of 25 kips per linear foot.
- Based on the results of the subsurface exploration and provided loads, the proposed 4-story parking structure and 15-story hotel tower will require deep foundation support. We recommend 16-inch or greater diameter auger-cast piles. The piles should be advanced to refusal in rock material. Capacities for 16-inch and 18-inch diameter piles bearing into rock are summarized as follows:

Allowable Capacity in Tons**					
Com	pression	Tension			
16-inch	16-inch 18-inch		18-inch		
110 150		25	40		

- Due to the depth of rock within the proposed parking deck (up to 127 feet), we recommend 18-inch diameter friction auger-cast piles with a minimum length of 105 feet. The 18-inch diameter friction piles will have 100 tons capacity for compression and 40 tons for tension.
- Deep zones of organic / unsuitable fill materials and boulders are anticipated to be encountered within the proposed single-story conference center. Indicated on the boring and test pit location plans included in the Appendix is a transition line between the clean fill materials and deep zones of organic / unsuitable fill materials. Single-story conference center building west of the transition line may be supported by a conventional shallow foundation system. Single-story conference center building structure (including slab area) that lies east of the transition line should be supported on an Aggregate Pier/ Grouted Pier System. Please refer to sections 6.1 through 6.3.
- For proposed pavement areas located east of the transition line, in-place stabilization of the exposed subgrade will be required. Please refer to section 5.1 for more information regarding subgrade stabilization.



2.0 INTRODUCTION

2.1 Site and Project Description

The project site lies within the eastern portion of the Avalon development located in Alpharetta, Georgia. More specifically, the Avalon development is located northwest of the intersection of Georgia Highway 400 and Old Milton Parkway.

Proposed for development is the construction of a Hotel and Conference Center with an approximate 97,500 square foot building footprint. The proposed hotel will be a fifteen-story tower with column loads ranging from approximately 300 to 1150 kips. The conference center will be a one-story structure with column loads ranging from approximately 25 to 175 kips. In addition to the Hotel and Conference Center, a four-story parking deck will be constructed. The proposed parking deck will have column loads ranging from approximately 275 kips to 750 kips and maximum wall loads of 25 kips per linear foot.

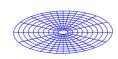
The proposed Hotel and Conference Center will have a finished floor elevation of 1064 feet. The majority of the Hotel and Conference Center structure is near or at proposed finished grade; however, to accommodate for the proposed Hotel and Conference Center building layout, the existing slope located on the eastern boundary of the site will require re-grading and construction of a 4 to 6 foot tall concrete retaining wall. The proposed parking deck will be partially underground and will require cuts up to 12 feet to achieve proposed finished grades.

2.2 Scope of Work

This report presents the results of our Geotechnical Exploration performed for the Avalon Hotel and Conference Center at the Avalon development in Alpharetta, Georgia. The purpose of this study was to provide a geotechnical exploration within the proposed development and determine the effects as they relate to the site development.

Our services were provided in generally accordance with the scope of services outlined in Contour's Proposal G14NAP-103RII, dated March 24, 2014. The services rendered by this firm included a site reconnaissance, drilling and sampling of seventeen (17) soil test borings, excavating eight (8) test pits, engineering analyses of obtained information, and preparation of this report. Specifically, our report addresses the following:

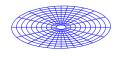
- Description of existing conditions including detailed records of the previous and new soil test borings and soil profiles and a Boring Location Plan,
- A description of the area and site geologic conditions,
- Determine groundwater elevations and provide recommendations for subsurface groundwater control,
- Recommendations for site preparation, excavation and grading, backfilling and compaction;
- Recommendations for subgrade preparation and slab-on-grade construction recommendations,
- Excavation conditions and the presence of very dense materials, partially weathered rock, or rock and the degree of difficulty of excavation,



- Recommendation for foundation design and construction including allowable bearing pressures and settlements,
- Lateral earth pressures for retaining walls;
- Recommendations for temporary and permanent slopes, and
- Seismic information based on the International Building Code 2012.

This report also includes information concerning subsurface exploration and soil test borings previously conducted and submitted to Prospect Park Partners North, LLC (c/o North American Properties, LLC) as the report titled "Final Report of Geotechnical Exploration, Avalon Development" dated May 1, 2012. More specifically, soil test borings H-1, H-2, H-3, H-4, Q-1, Q-2, Q-3, Q-4, P-37, P-38, P-39, P-45, and P-47 and test pits TP-10 through TP-12 and TP-17 through TP-19 have been included from the previous report.

The scope of our services did not include any environmental assessment or exploration for the presence or absence of hazardous or toxic materials in the soil, groundwater, or surface water within or beyond the site.



3.0 FIELD EXPLORATION AND LABORATORY PROGRAM

3.1 Field Exploration

Our field exploration consisted of a site reconnaissance and performing a total of seventeen (17) soil test borings designated as C-1 through C-17. More specifically, eleven (11) soil test borings designated as C-1 through C-11 were performed within the proposed Hotel and Conference Center. Borings C-1 though C-9 extended to planned termination depths of 70 feet; while borings C-10 and C-11 extended to auger refusal depths of 71 and 76 feet, respectively.

Five (5) soil test borings designated as C-12 through C-16 were performed within the proposed parking deck. Borings C-12, C-14, and C-15 extended to planned termination depths of 70 feet; while borings C-13 and C-16 extended to auger refusal depths of 127 and 92 feet, respectively. One (1) soil test boring C-17 was performed within proposed parking and drive area and extended to planned termination depth of 70 feet.

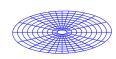
In addition to the soil test borings, eight (8) test pits were excavated within the proposed Hotel and Conference Center. Designated as TP-20 through TP-27, the test pits were excavated to depths of 15 feet below existing ground surface.

The soil test boring and test pit locations were determined utilizing and Ashtech GPS receiver in the field by Contour personnel. Therefore, the boring locations should be considered accurate. The boring locations are shown on the attached Boring Location Plan in Appendix.

The sampling and penetration procedures of the soil test borings were performed in accordance with ASTM D-1586, using a power rotary drill. The standard penetration tests were performed by driving a standard 1-3/8" I.D. and 2" O.D. split spoon sampler with an automatic 140-pound hammer falling 30 inches. The number of hammer blows required to drive the sampler a total of 18 inches, in 6-inch increments, were recorded. The penetration resistance or "N" value is the summation of the last two 6-inch increments and is illustrated on the attached boring logs adjacent to their corresponding depths. In very dense soils or weathered rock, the sample is driven a few inches rather than the 6-inch increment and the number of blows required versus the penetration depth is recorded. The penetration resistance is used as an index to derive soil parameters from various empirical correlations.

3.2 Laboratory Program

A representative portion of each recovered sample was sealed in a glass container and transported to our laboratory for further visual classification (ASTM D-2487). Using the Unified Soil Classification system, the subsoil conditions are described and stratified in an illustrated form of soil profiles on the attached boring logs.



4.0 SITE AND SUBSURFACE CONDITIONS

4.1 Area Geology

Published information concerning the geology of the area indicates that the site is located in the Piedmont Geologic Region, a broad northeasterly trending province underlain by crystalline rocks up to 600 million years old. The Piedmont is bounded on the northwest by the Blue Ridge Range of the Appalachian Mountains, and on the southeast by the leading edge of Coastal Plain sediments, commonly referred to as the "Fall Line". Numerous episodes of crystal deformation have produced varying degrees of metamorphism, folding and shearing in the underlying rock. The resulting metamorphic rock types in this area of the Piedmont are predominantly a series of Precambrian age schists and gneisses, with scattered granitic or quartzite intrusions.

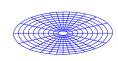
The Piedmont consists of surficial soils that are the residual products of the in-place weathering of the parent rock. The residual soils are sometimes overlain by alluvial soils, which were transported and deposited by flowing water, or by man placed filled materials. The underlying rocks are primarily metamorphic gneiss, schist, and granite. The residual soils are generally clayey silts near the ground surface underlain by sandy silts and silty sands.

The closest Fault Zone to the site is the Brevard Fault Zone, which is an inactive Fault.

4.2 Soil Survey

According to the Natural Resource Conservation Service (NRCS) <u>On-line Soil Survey of Fulton County, Georgia</u>, the following soil series were identified within the site location: Cecil sandy loam, 6 to 10 percent slopes, moderately eroded (CeC2), Grover-Mountain Park complex, 20 to 60 percent slopes, stony (GaF), and Pacolet-Saw Complex, 6 to 10 percent slopes (PgC2). The soil series identified are further described as follows:

- The <u>Cecil Series</u> consists of very deep, well-drained moderately permeable soils on ridges and side slopes of the Piedmont uplands. They are deep to saprolite and very deep to bedrock. They formed in residuum weathered from felsic, igneous and high-grade metamorphic rocks of the Piedmont uplands.
- The <u>Grover Series</u> consists of very deep, well drained soils on ridges and side slopes on Piedmont uplands. They formed in residuum that is affected by soil creep in the upper part on steep slopes, and is weathered from high-grade metamorphic rocks high in mica such as biotite gneiss and schist.
- The <u>Mountain Series</u> consists of moderately deep, well drained, moderately permeable soils on Piedmont uplands. They formed in residuum that is weathered from high-grade metamorphic rocks high in mica such as biotite gneiss and mica schist.
- The <u>Pacolet Series</u> consists of very deep, well drained, moderately permeable soils that formed in residuum weathered mostly from felsic igneous and metamorphic rocks of the Piedmont uplands.



• The <u>Saw Series</u> consists of moderately deep, well drained soils on ridges and side slopes of the Piedmont uplands. They formed in residuum weathered from felsic igneous rocks such as porphyritic granite and granite. Slopes range from 2 to 45 percent.

4.3 Subsurface Soil Conditions

Hotel and Conference Center

Below the ground surface, the soil test borings encountered fill materials, residual soils, partially weathered rock and auger refusal. **Fill materials**, soils that have been transported and placed by man, were encountered in all of the borings performed within the proposed Hotel and Conference Center. The fill materials extended to depths ranging from 17 to 45 feet below existing ground surface. Borings H-2, H-3, P-37, P-47 and Q-4 were terminated in fill materials at depths of 20 feet. Boulder fill was initially encountered in boring C-3 at an approximate depth of 13 feet which resulted in auger refusal. Boring C-3A was offset approximately 20 feet east of the original location and was able to penetrate through the fill material.

Residual soils, soils formed by in-place weathering of the parent rock, were encountered beneath the fill materials and extended to planned boring termination depths of 20 and 70 feet or its interface with partially weathered rock. The sampled residual soils were classified as silty sands (SM) and sandy silts (ML) with Standard Penetration Test (SPT) values ranging from 5 to 80 blows per foot (bpf).

Partially weathered rock (PWR), locally defined as very dense soils or highly weathered rock with penetration values of more than 100 blows per foot, was encountered in borings C-10 and C-11 at depths of 65 and 70 feet respectively, corresponding elevations of 1001 and 995 feet.

Auger refusal material (rock), material that cannot be penetrated any further by the power auger, was encountered in borings C-10 and C-11 at depths of 71 and 76 feet, corresponding elevations of 995 and 989 feet.

Parking Deck

Below the ground surface, the soil test borings encountered fill materials, residual soils, partially weathered rock and auger refusal. **Fill materials** were encountered in all of the borings performed within the proposed parking deck. The fill materials extended to depths ranging from 7 to 40 feet below existing ground surface.

Residual soils were encountered beneath the fill materials and extended to planned boring termination depths of 20, 35, and 70 feet or its interface with partially weathered rock or auger refusal material. The sampled residual soils were classified as silty sands (SM) and sandy silts (ML) with SPT values ranging from 6 to 37 bpf.

Partially weathered rock was encountered in boring C-16 at a depth of 85 feet, corresponding elevation of 975 feet.



Auger refusal material (rock) was encountered in borings C-13 and C-16 at depths of 127 and 92 feet, corresponding elevations of 939 and 968 feet.

South Parking Area

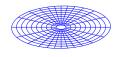
Below the ground surface, the soil test borings encountered fill materials and residual soils. **Fill materials** were encountered in all of the borings performed within the proposed parking deck. The fill materials extended to depths ranging from 20 to 37 feet below existing ground surface. Boring P-45 was terminated in fill materials at a depth of 20 feet.

Residual soils were encountered beneath the fill materials and extended to planned boring termination depths of 40 and 70 feet. The sampled residual soils were classified as silty sands (SM) and sandy silts (ML) with SPT values ranging from 8 to 54 bpf.

Individual soil boring profiles are depicted on the Boring Log Records included in the Appendix. Three (3) Subsurface Profile Plates illustrating the subsurface surface conditions encountered within the proposed Hotel and Conference Center and parking deck including fill materials, residual soils, PWR, auger refusal material, and groundwater conditions are included in Appendix. The stratification lines indicated on the Boring Log Records and Subsurface Profile Plates represent the approximate boundaries between soil types. The actual transitions between soil strata may be gradual.

4.4 Groundwater Conditions

The measurement to the depth below the existing ground surface to the groundwater table was attempted immediately following and 24 hours after the completion of each boring. Groundwater was encountered at depths varying from 46 to 60 feet below ground surface. The groundwater levels in this area will fluctuate in response to local variations of precipitation and temperature and may be different at other times and areas.



5.0 EARTHWORK RECOMMENDATIONS

5.1 Site Preparation

Unsuitable materials containing topsoil and organics were used to construct portions of the existing slope located east of the proposed Hotel and Conference Center. Test pits were excavated along the slope to delineate the unsuitable materials. The location of the test pits are shown on the attached Test Pit Location Plan included in the Appendix. An approximate transition line between suitable fill materials and unsuitable organic laden material is shown on the Test Pit and Boring Location Plans. Photographic documentation of the excavated test pits is also included in the Appendix.

For proposed pavement areas located east of the transition line, we recommend evaluating the subgrade and if found unstable or underlain with unsuitable fill materials, undercut subgrade to depths of 7 feet below finished subgrade and then stabilize the exposed subgrade. We envision in-place stabilization will consist of either of a soil bridge lift (if exposed subgrade conditions allow during construction) or placement of a stabilization fabric overlain by 2 feet of crushed stone. Once the exposed subgrades are stabilized in-place, structural fill should be placed and compacted to proposed grades in accordance with the project specifications.

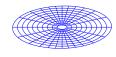
For slab and foundation subgrade support located east of the transition line please refer to sections 6.1 through 6.3.

For areas of west of the transition line, we recommend a geotechnical engineer carefully evaluate the areas intended to support floor slabs, pavements, new fill, and foundations. At that time, the engineer may require proofrolling of the subgrade with a 20 to 30-ton loaded tandem-axle dump truck or other pneumatic-tired vehicle of similar size and weight. The purpose of the evaluation is to locate soft, weak, or excessively wet soils present at the time of construction. Any unsuitable materials observed during the evaluation and/or proofrolling operations should be undercut and replaced with compacted fill or stabilized in-place.

5.2 Excavation Conditions

PWR and auger refusal material was encountered within the proposed Hotel and Conference Center at elevations ranging from 1001 to 995 feet for PWR and 995 to 989 feet for auger refusal material. Based on provided grading information, PWR and auger refusal material was not encountered above proposed finished floor elevation of 1064 feet.

PWR and auger refusal material was encountered within the proposed parking deck at an elevation of 975 feet for PWR and elevations ranging from 968 to 975 feet for auger refusal material. Based on provided grading information, PWR and auger refusal material was not encountered above proposed finished floor elevation of 1054 feet.



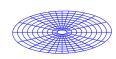
5.3 Structural Fill

Based on the boring and laboratory data, the encountered residual soils and existing fill material free of organics appear suitable for reuse as structural fill. Off-site borrow material may also be used as structural fill provided that they have a liquid limit (LL) and a plastic index (PI) not exceeding 40 and 20 percent, respectively. All structural fill should be moisture conditioned to maintain a moisture content within two percentage points above and below the soil's optimum moisture content as determined by the Standard Proctor test (ASTM D-698). Therefore, the grading contractor should be prepared to moisture condition the soil as required during fill placement.

Structural fill should be placed in thin loose lifts not exceeding 8 inches in thickness and compacted accordingly. A Contour Engineering soils technician should test any new fill to determine the compaction percentage. Field density testing should be performed as one test per 2-foot lift for every 5,000 square feet in the building areas and 10,000 square feet in the pavement areas.

Based on our experience with soils similar to those on this site and similar types of construction, we recommend that the following minimum level of compaction be achieved:

- Building Areas and Interior Slabs 98 percent of the soil's maximum standard Proctor density value (ASTM D-698) or 95 percent of soil's maximum Modified Proctor (ASTM D-1557). In cut areas, the subgrade should be proofrolled and if found unstable, it should be scarified and re-compacted to 98 percent of the soil's maximum standard Proctor density value.
- Pavement Areas Compact the upper 18 inches of subgrade in fill areas and the upper 12 inches in cut areas to 98 percent of the soil's maximum standard Proctor density value (ASTM D-698) or 95 percent of soil's maximum Modified Proctor (ASTM D-1557) prior to placement of the base course material and 95 percent of the soil's maximum standard Proctor density value below this level. In cut areas, the subgrade shall be proofrolled and if found unstable, should be scarified and re-compacted to 98 percent of the soil's maximum standard Proctor density value.
- Utility Trenches Compact the upper 18 inches of the subgrade to 98 percent of the soil's maximum standard Proctor density value (ASTM D-698) or 95 percent of soil's maximum Modified Proctor (ASTM D-1557) and 95 percent of the soil's maximum standard Proctor density value below this level.
- Landscape Areas 92 percent of the soil's maximum standard Proctor density value (ASTM D-698).
- Sidewalks, Exterior Slabs, Stormwater Pond Embankments Compact the upper 24 inches of subgrade in fill areas and the upper 12 inches in cut areas to 98 percent of the soil's maximum standard Proctor density value (ASTM D-698) or 95 percent of

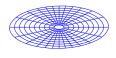


soil's maximum Modified Proctor (ASTM D-1557) and 95 percent of the soil's maximum standard Proctor density value below this level

5.4 Groundwater and Drainage Considerations

Groundwater was encountered at varying depths from 46 to 60 feet below ground surface at the time of drilling. Based on provided grading information, groundwater is not anticipated to be encountered during mass grading operations. Utility plans were not available for our review. Should groundwater be encountered during remedial work or utility installations, we recommend that the contractor implement dewatering techniques to maintain groundwater levels a minimum of 36 inches below working subgrades.

Excessive moisture from rain runoff can significantly reduce the soil's bearing capacity and contribute to foundation settlement as well as pavement failure. Positive drainage should be provided around the perimeter of the buildings to reduce the infiltration of water. All grades should be sloped away from the building. Finger drains at catch basins and draintile at irrigated landscaped islands are recommended.



6.0 DESIGN RECOMMENDATIONS

6.1 Foundation Support Recommendations

Based on the results of the subsurface exploration and provided loads, the proposed 4-story parking structure and 15-story hotel tower will require deep foundation support. The single-story conference center may be supported on shallow foundation systems bearing on existing fill materials or improved subgrade soils.

A transition line as shown on the attached boring and test pit location plans delineates the approximate transition of the clean fill materials and deep zones of organic / unsuitable fill materials and boulders. Single-story conference center building west of the transition line may be supported by a conventional shallow foundation system. Single-story conference center building structure (including slab area) that lies east of the transition line should be supported on an Aggregate Pier/ Grouted Pier System. A contractor specializing in Pier Foundations should be consulted for the design of this foundation system.

Single-Story Buildings

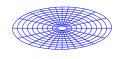
Shallow Footing Recommendations

Based on the results of the subsurface exploration and anticipated loads, the proposed single-story buildings (located west of the transition line) may be supported on a conventional shallow foundation system. Provided that the foundations are prepared in accordance with our recommendations, a maximum allowable bearing pressure of 3,000 pounds per square foot may be used in the design of the shallow foundation system parking structure and Ball Room.

To reduce the possibility of shear failure, wall bearing and column foundations should be designed with a minimum width of 18 and 24 inches, respectively. For frost protection, exterior wall bearing and column foundations should be designed with a minimum embedment depth of 18 inches, while interior foundations should be designed with a minimum embedment depth of 12 inches. The embedment depth should be measured from the base of the foundation to lowest adjacent outside grade.

Bottoms of foundation excavations should be evaluated by a geotechnical engineer prior to placement of reinforcing steel and concrete to verify that adequate bearing materials are present and that all debris, mud, and loose, frozen or water-softened soils are removed.

Foundation excavations should be concreted as soon as practical after they are excavated. Water should not be allowed to pond in any excavation. If an excavation is left open for an extended period, a thin mat of lean concrete should be placed over the bottom to minimize damage to the bearing surface from weather or construction activities. Foundation concrete should not be placed on frozen or saturated subgrades.



Hotel Tower & Parking Deck

Auger-Cast Piles- Design

Auger-cast piles may be used to support the parking deck and 15-story building foundations. Due to the presence of boulder fill, some piles may not penetrate through the fill and will require replacement. Premature refusal will be a field determination based on assumed depth of fill material and refusal criteria. The piles will be abandoned and replaced. A new pile configuration will be provided to the structural engineer for redesign of pile cap.

For the Hotel and Conference Center, we recommend 16-inch or greater diameter auger-cast piles. The piles should be advanced to refusal in rock material. Refusal should be defined as a penetration rate of one foot (or less) per minute using a drive box having a minimum dead weight of 5000 lbs. and a torque of 20,000 foot-pounds. Capacities for 16-inch and 18-inch diameter piles bearing into rock are summarized as follows:

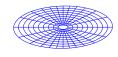
	Allowable Ca	pacity in Tons**	
Com	pression	Te	nsion
16-inch	18-inch	16-inch	18-inch
110	150	25	40

The selection of pile diameter should be based on structural loads and economics. Minimum center-to-center auger-cast pile spacing should be at least three (3) pile diameters to reduce pile group settlements and minimize axial capacity reductions due to group effects. We anticipate that pile tip depths will be to within a few feet of our boring refusal elevations. Only two borings refused at depths of 71 and 76 feet in the Hotel and Conference Center. The rock surface is irregular and additional borings will be required to more accurately determine refusal depths of the piles.

Due to the depth of rock in the proposed parking deck (up to 127 feet), 18-inch diameter friction auger-cast piles with a minimum length of 105 feet may used to support the structure. The 18-inch diameter friction piles will have 100 ton capacity for compression and 40 tons for tension.

We have preliminarily evaluated lateral load resistance for single piles under a fixed-head condition. The following table summarizes lateral capacities based on an assumed pile top lateral deflection of 1/4 inch:

Preliminary Pile L	ateral Load Analysis
Pile Diameter (inches)	Lateral Load (kips) to Cause Pile Top Deflection of 1/4 inch
16	10
18	15



Auger-Cast Pile-Installation

The success of auger-cast pile construction is highly dependent on the skill of the piling contractor. Pile installation should be monitored by the geotechnical engineer. The grout pump should be calibrated prior to initiation of production piles, and as often as necessary throughout the installation of piles as deemed necessary by the engineer. The minimum grout head or grout return depth should be 5 feet. The grout filling operation should continue without interruption until the auger is extracted completely from the ground. Auger withdrawal during the grouting process should be at a constant rate and be completed in a continuous process. We recommend that the rate of auger withdrawal during grouting be coordinated such that a minimum of 115 percent of the theoretical pile volume is pumped into each 5-foot increment of length of pile. If the grouting process is interrupted, the pile should be reaugered at least 5 feet before grouting is continued. If the process is interrupted for more than 15 minutes, the pile should be completely reaugered and regrouted. Pile installation should be no closer than 3 pile diameters until the adjoining pile's grout has obtained its initial set or within 24 hours, whichever is greater.

The piles should be load tested prior to the installation of the production piles to verify the actual pile capacities. The load test should be performed in accordance with ASTM D-1143 under the observation of the geotechnical engineer. We also recommend that 15 probe piles be installed across the site prior to commencement of the load tests.

6.2 Floor Support

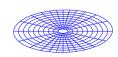
The building slabs located on the west of the transition line may be supported directly on stable soil subgrades. A modulus of subgrade reaction of 100 pounds per cubic inch (pci, pounds per square inch per inch of deflection) will be available if the slab is placed directly on stable soil subgrade.

If a higher modulus of subgrade reaction is required; then we recommend that a 4 to 6-inch layer of compacted crushed stone be placed underneath the building slab. The 4 to 6 inches of crushed stone will provide a protective cover as well as a uniform working surface and also serve as a capillary break. The crushed stone should consist of crushed aggregate base meeting the requirements of GDOT Section 815. Slabs underlain by 4 to 6 inches of stone will have moduli of subgrade reactions (K) of 130 and 150 pci, respectively.

Expansion and contraction joints should be used to isolate all floor slabs from the load bearing walls and/or isolated columns. This will allow for possible differential movement and diminish the potential of cracking the floor slabs.

6.3 Aggregate / Grouted Pier Foundation Systems

Aggregate Pier or Grouted Pier foundation systems are recommended for the support of the single-story buildings and slabs (including the slab for the 15-story building) located east of the transition line (see attached site plan delineating areas of unsuitable fill materials), which are underlain by deep zones of organic / unsuitable fill materials and boulders. Aggregate



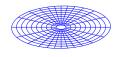
Pier/Grouted Pier foundations are proprietary systems; therefore, the remedial work for support of the foundations and slab systems will be performed as a design/build application. If this is determined to be a feasible option by the design/build contractor, the design engineer of record for the Aggregate Pier/Grouted Pier foundation system should take the following into account during the design and construction phases:

- Our borings encountered unsuitable fill depths ranging from 3 to 40 feet below existing grades;
- The alternative Foundation System designed to support building slabs and foundations should be constructed at a minimum of 2 feet below building subgrades to eliminate point loading;
- The design/build contractor or general contractor should include provisions in their bids to stabilize the subgrade, if necessary, after completion of the installation of the Foundation System;
- The fill placed over the selected Foundation System should consist of structural fill or stone compacted to 98% of standard Proctor (ASTM 695);
- The Pier foundation system should be designed for total settlements of 1-inch or less with differential settlements of ½ inch in 40 feet of length, and a minimum subgrade modulus of 100 PCI; and
- Upon completion of the Aggregate / Grouted pier Foundation System installation, the design/build contractor and the design engineer of record should provide a pad and foundation certification letter signed by the design engineer of record confirming that the above-mentioned design criteria have been met.

We recommend that the above mentioned design criteria be included in the design/build contractor's contract. We appreciate the opportunity to be of service on this project. Should you have any questions regarding this report or if we may be of further service, please contact our office.

6.4 Seismic Recommendations

Based on the results of our boring data, published geological features of the project area, the Site data, and as described per Section 1613.5.2 of the 2012 International Building Code (IBC), it is our opinion that the Seismic Site Class Definition for the Site is "D". The soil profile named "Stiff Soil Profile" was determined from Table 1613.5.2 of the 2012 IBC.



6.5 Concrete and/or Below Grade Retaining Wall Design

It is our understanding that a concrete retaining wall will be constructed along the eastern boundary. Unsuitable materials may be encountered within the existing slope. If encountered during construction of the retaining wall, we recommend undercutting the unsuitable materials down to residual soils. The removal of the unsuitable material should be removed from the face of the wall and extend to a distance behind the wall equal to the height of the retaining wall. All unsuitable materials should be removed beneath the retaining wall footing. A Retaining Wall Detail is included in the Appendix illustrating recommended removal of unsuitable materials if encountered.

The loading dock walls, concrete retaining walls and any other on-site concrete below grade walls will be subjected to lateral earth pressures. Walls that are relatively rigid or fixed at the top and bottom may be subjected to "at-rest" earth pressures. Walls that are allowed to have sufficient movement and not fixed at the top will be subjected to "active" pressures.

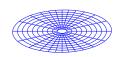
The following lateral earth pressure parameters are recommended for design in residual soils or structural fill. Fill materials with organic contents in excess of 3 percent are not suitable for reuse as select backfill for placement behind retaining walls.

Earth Pressure Coefficient	Earth Pressure Value	Equivalent Fluid Density (pcf)
At-Rest (K _O)	0.53	64
Active (K _A)	0.36	43
Passive (K _P)	2.77	332

These values assume that the wall has horizontal backfill and no surcharge loads such as from adjacent structures. A moist unit weight of 120 pounds cubic foot, a phi angle of 28 degrees and a sliding coefficient of 0.53 may be used in the *ultimate design value* of retaining walls. Typically, a factor of safety of 1.5 is used for the passive earth pressure and coefficient of friction.

The recommended equivalent fluid pressures assume that constantly functioning drainage systems are installed between walls and soil backfill to prevent the accidental buildup of hydrostatic pressures and lateral stresses in excess of those stated. If a functioning drainage system is not installed, then lateral earth pressures should be determined using the buoyant weight of the soil (approximately 58 pcf). Hydrostatic pressures calculated with the unit weight of water (62.4 pcf) should be added to these earth pressures to obtain the total stresses for design.

To facilitate drainage behind retaining walls / below grade walls, we recommend the use of weep holes/perforated pipe encased #57 stone (wrapped in Geotextile Filter Fabric, such as Mirafi 140N) and granular backfill. More specifically, the granular backfill to be used as free draining material shall consist of clean 1-inch crushed stone or gravel meeting the following gradation:



Granular Backfill G	radation Requirements
Sieve Size	Percent Passing
4-inch	100
³ ⁄ ₄ -inch	75 - 100
No. 4	0 - 10
No. 50	0 -50

A Typical Backfill/Retaining Wall Drainage Detail is included in Appendix of this report.

The surcharge and lateral loads from tractors and other heavy equipment operating within 10 feet of below grade walls should be added to the lateral loads cited in this section of the report. If foundations or other surcharge loadings are located a short distance outside below grade walls, they may also exert appreciable additional lateral pressures that must be considered.

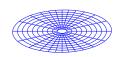
The retaining wall/below grade wall recommendations listed above should not be correlated with soil parameters for use in Segmental Retaining Wall design and/or Reinforced Slope Design.

6.6 Slope Recommendations

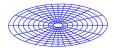
Temporary slopes not exceeding 10 feet in height for confined areas and constructed in the virgin soils or structural fill, should be configured no steeper than 1.5(H):1.0(V) provided no water is observed seeping from the sides of the excavation. These temporary slopes should be regularly monitored for signs of movement or unsafe conditions. Temporary slopes below the groundwater table will require shoring / bracing. Additionally, construction excavation should comply with OSHA Guidelines outlined in the Code of Federal Regulations Federal Register Volume 54, Number 209 (October 1989) "Construction Standards for Excavation, 29CFR Part 1926, Subpart P." Also, the contractor should have a designated "qualified engineer" as defined by OSHA on-site during the excavation to observe the slopes for signs of possible failure.

Proper management of groundwater seepage and surface water runoff around the excavations will also contribute to the stability of temporary slopes. Material removed from excavations should *not* be stockpiled within a distance of twenty (20) feet from the crest of temporary excavations. Furthermore, positive drainage should also be maintained with ditches or channels at the top and bottom of the slope. It is also very important to always keep these drainage channels free of dirt, debris and vegetation.

The existing slope located east of the proposed Hotel and Conference will be re-graded to allow for the construction of the proposed structures. It is critical that these fill soils are benched into the existing slopes in accordance with the Typical Benching Detail included in Appendix. Due to the presence of topsoil and organic laden material within the existing slope, we recommend that the slope be re-graded no steeper than 3.0 (H): 1(V) configuration. To prevent erosion and saturation of the slopes, surface runoff water should be diverted from the top of the slopes. A protective cover of grass or other vegetation should be established on the slopes as soon as possible for erosion protection. Depending on site conditions, a toe drain may also be required at the toe of cut slopes to collect water seepage.



Buildings should have a minimum setback of 10 feet from the slope shoulders. A minimum setback of 5 feet is recommended for the pavement curbs.



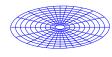
7.0 QUALIFICATION OF RECOMMENDATIONS

This report has been prepared based on currently accepted geotechnical engineering principles and practices in the local area for the specific application of this project.

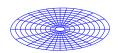
The analyses and recommendations presented in this report are based upon our understanding of the site and project information and the data obtained from our field exploration. If there are any revisions to the plans for this project, we should be permitted to determine if the recommendations must be modified. The nature and extent of variations between borings will not be evident until the course of construction; if such variations become evident, it may be necessary to submit supplementary recommendations.

Regardless of the thoroughness of a geotechnical study, there is always a possibility that subsurface conditions will be different from those at the boring locations that conditions will not be as anticipated by the designers or contractors, or that the construction process will alter soil conditions. Therefore, the geotechnical engineer's representative should observe and confirm that the conditions indicated by the geotechnical exploration actually exist.

Once final design plans and specifications are complete, we recommend that Contour Engineering, LLC be provided the opportunity to review the final design and specifications in order that earthwork and foundation recommendations are properly interpreted and implemented.



APPENDIX





SITE VICINITY MAP



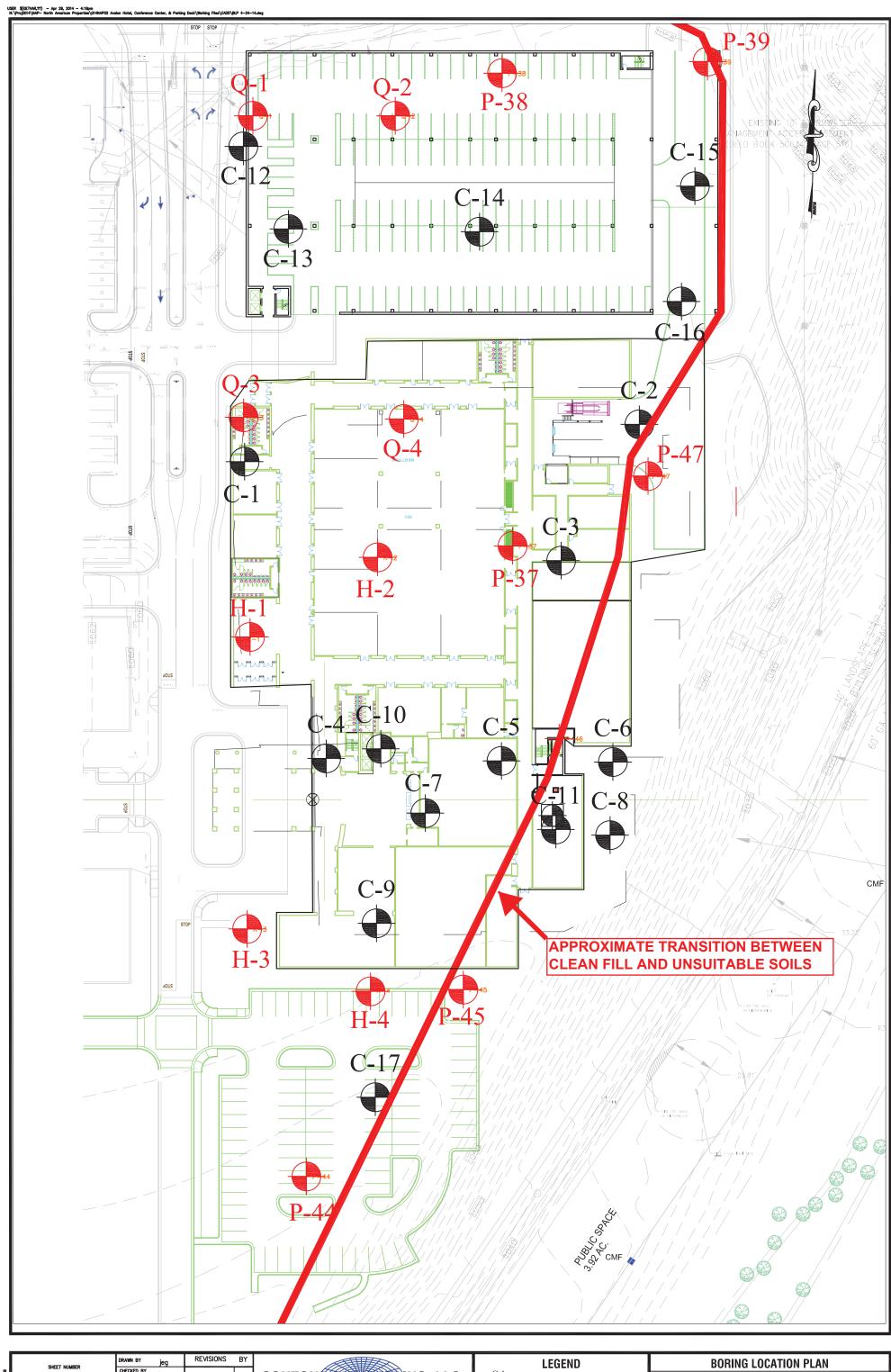
LEGEND

Source: GoogleEarth.com Scale: Unknown

PROJECT

Geotechnical Exploration

Hotel and Conference Center - Avalon
Alpharetta, Fulton County, Georgia
Contour Project: G14NAP02



BLP-1 1 OF 1 SHEET

jmr DATE: 04/29/2014 1"= 60' PROJECT No. G14NAP02

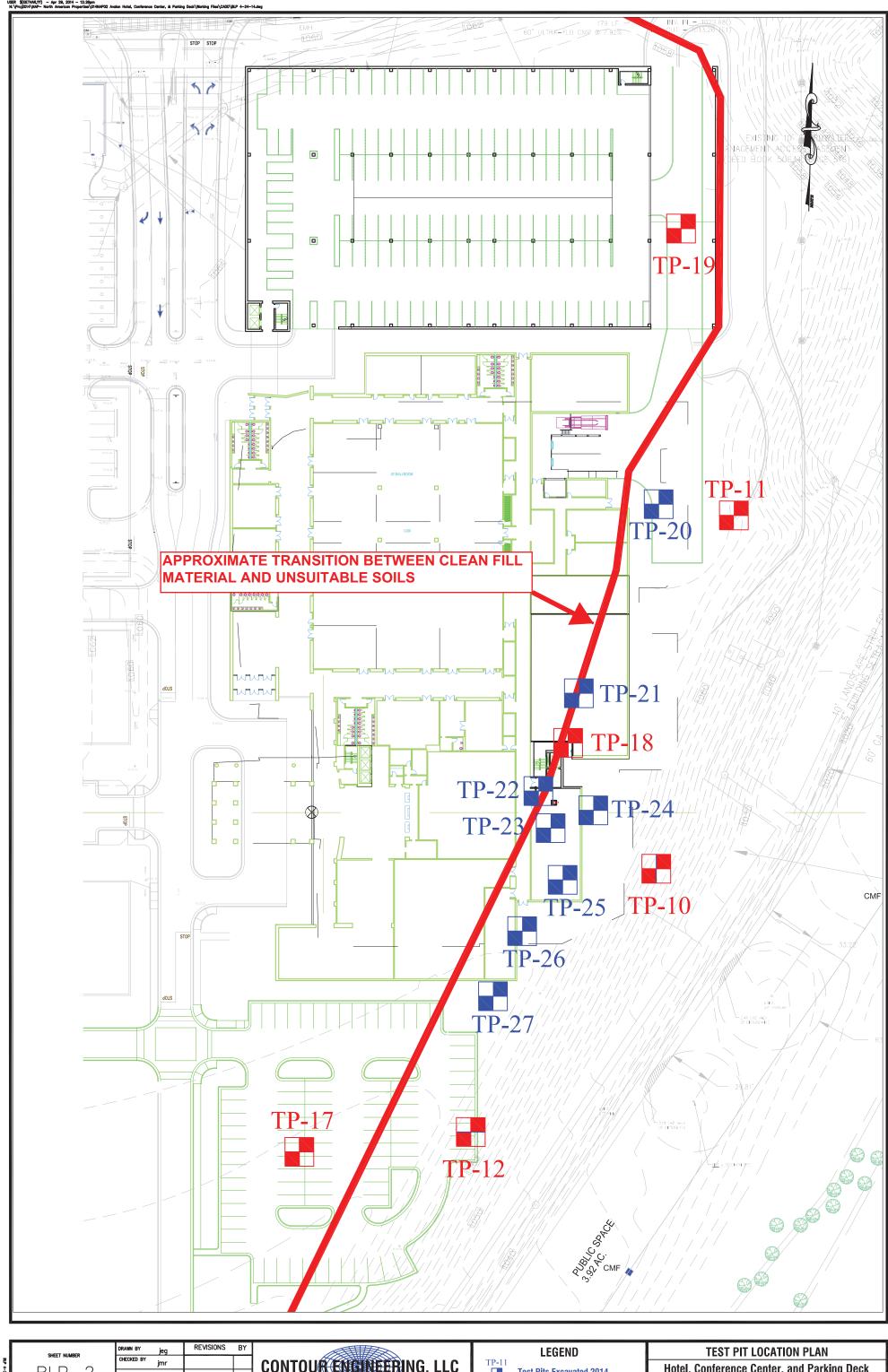
CONTOUR ENGINEERING, LLC

1955 Vaughn Road, Sulte 101, Kennesaw, GA 30144 Phone: (770) 794-0266 Fax: (770) 794-9483

- Soil Test Boring Performed 2014

- Soil Test Boring Performed 2012

Hotel, Conference Center, and Parking Deck Alpharetta, Fulton, Georgia Project No: G14NAP02



BLP-21 OF 1 SHEET

DATE: 04/29/2014 SCALE: 1"= 60' PROJECT No. G14NAP02

CONTOUR ENGINEERING, LLC

1955 Vaughn Road, Sulte 101, Kennesaw, GA 30144 Phone: (770) 794-0266 Fax: (770) 794-9483

- Test Pits Excavated 2014

- Test Pits Excavated 2012

Hotel, Conference Center, and Parking Deck Alpharetta, Fulton, Georgia Project No: G14NAP02

Avalon Phase II

Test Pits Alpharetta, Fulton County, Georgia Contour Project No: G14NAP02



Photograph 1
Test Pit TP-20.
Top soil encountered between -5 to -6 feet below current elevation.



Photograph 2
Spoils from TP-20



Photograph 3
TP-21. No top soil encountered in 15 feet below current elevation.



Photograph 4
Spoils from TP-21

CONTOUR ENGINEERING, LLC

Avalon Phase II

Test Pits Alpharetta, Fulton County, Georgia Contour Project No: G14NAP02



Photograph 5
Test Pit TP-22.
No top soil encountered in 15 feet.



Photograph 6 TP-22 Spoils.



Photograph 7
Test Pit TP-23.
Top soil encountered between 7 to 8 feet and 9 to 10 feet below current elevation.



<u>Photograph 8</u> Organics found from TP-23.

CONTOUR ENGINEERING, LLC

Avalon Phase II

Test Pits Alpharetta, Fulton County, Georgia Contour Project No: G14NAP02



Photograph 9

Test Pit TP-24. Top soil encountered at -5' below current elevation through bottom of test pit at -15'.



Photograph 10
Test Pit TP-24 excavated soils.



Photograph 11
Test pit TP-25. Top soil encountered at 6' below current elevation through bottom of test pit at -15'.



<u>Photograph 12</u> Test Pit TP-25 excavated soils.

CONTOUR ENGINEERING, LLC

Avalon Phase II

Test Pits Alpharetta, Fulton County, Georgia Contour Project No: G14NAP02



Photograph 13

Test Pit TP-26. Top soil encountered at 8' below current elevation through bottom of test pit at -15'.

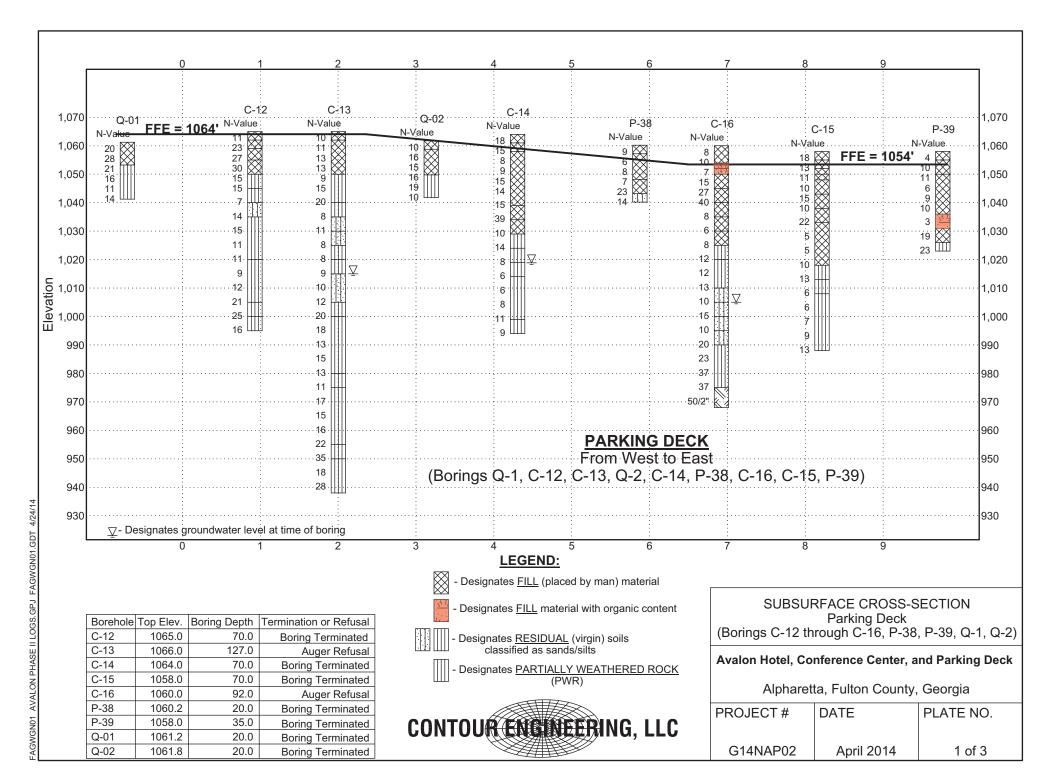


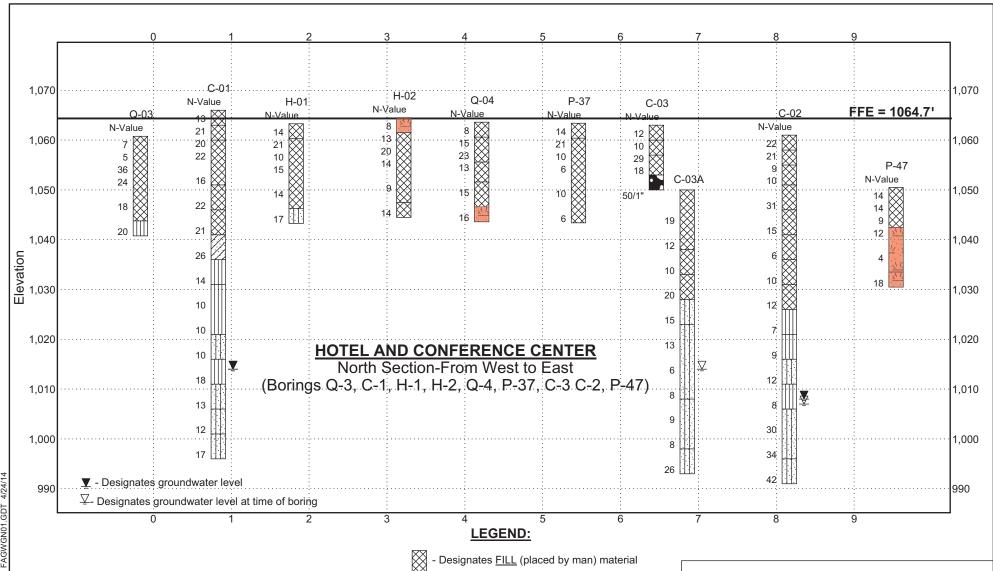
<u>Photograph 14</u> Test Pit T-26 excavated soils.



Photograph 15
Test Pit T-27.
Top soil encountered at 6' below current elevation through bottom of test pit at -15'.







Borehole	Top Elev.	Boring Depth	Termination or Refusal
C-01	1066.0	70.0	Boring Terminated
C-02	1061.0	70.0	Boring Terminated
C-03	1063.0	13.0	Auger Refusal
H-01	1063.3	20.0	Boring Terminated
H-02	1064.5	20.0	Boring Terminated
P-37	1063.4	20.0	Boring Terminated
P-47	1050.5	20.0	Boring Terminated
Q-03	1060.8	20.0	Boring Terminated
Q-04	1063.6	20.0	Boring Terminated

- Designates FILL material with organic content

- Designates <u>RESIDUAL</u> (virgin) soils classified as sands/silts

- Designates <u>BOULDER FILL</u> (placed by man)

CONTOUR ENGINEERING, LLC

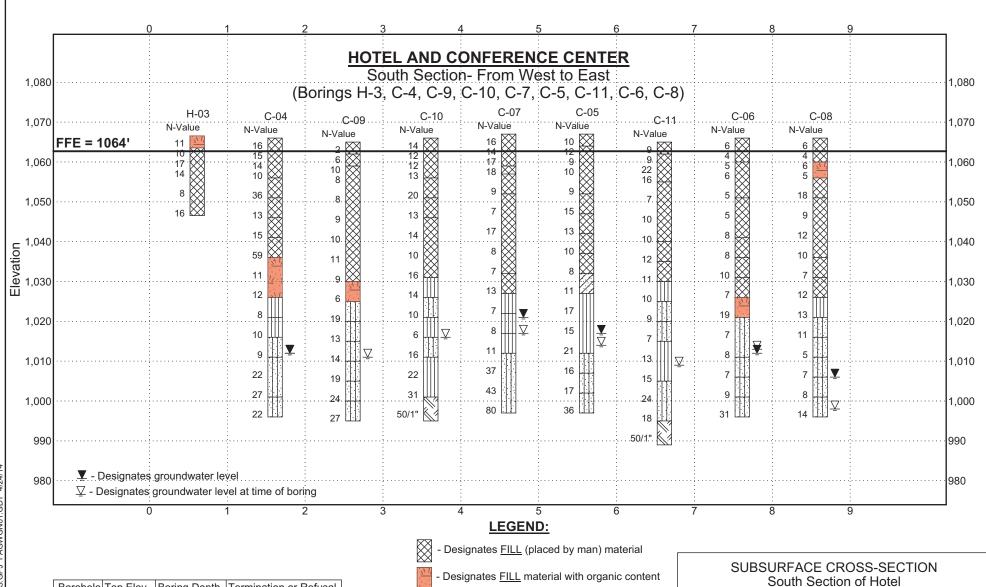
SUBSURFACE CROSS-SECTION North section of Hotel (Borings C-1, C-2, C-3, H-1, H-2, P-37, P-47, Q-3, Q-4)

Avalon Hotel, Conference Center, and Parking Deck

Alpharetta, Fulton County, Georgia

PROJECT# DATE PLATE NO.

G14NAP02 April 2014 2 of 3



Borehole	Top Elev.	Boring Depth	Termination or Refusal
C-04	1066.0	70.0	Boring Terminated
C-05	1067.0	70.0	Boring Terminated
C-06	1066.0	70.0	Boring Terminated
C-07	1067.0	Boring Terminated	
C-08	1066.0	70.0	Boring Terminated
C-09	1065.0	70.0	Boring Terminated
C-10	1066.0	71.0	Auger Refusal
C-11	1065.0	76.0	Auger Refusal
H-03	1066.6	20.0	Boring Terminated

Designates <u>RESIDUAL</u> (virgin) soils classified as sands/silts

- Designates PARTIALLY WEATHERED ROCK

CONTOUR ENGINEERING, LLC

South Section of Hotel (Borings C-4 through C-11, H-3)

Avalon Hotel, Conference Center, and Parking Deck

Alpharetta, Fulton County, Georgia

PROJECT#	DATE	PLATE NO.
G14NAP02	April 2014	3 of 3
GIANAPUZ	April 2014	3 01 3

LOGGI	ED BY: I	3lake	Summers	ELEV	ATION:	1066.0) ft								
			l 12, 2014			PTH: 70									_
DRILLI	NG ME	THOD:	Hollow Stem Auger	EL: 52.	0 ft				EVEL:	52.0	ft	_			
DEPTH (feet)	ELEVATION (feet)	GRAPHIC LOG	SAMPLE DESCRIPTION	NATURAL MOISTURE CONTENT (%)	LIQUID LIMIT (%)	PLASTIC INDEX (%)	BLOW COUNTS	1			DATA ue, blo			0 1	
5.0	1063.0		FILL: Brown-pink, silty medium SAND (SM), trace rock fragments Brown-pink, silty coarse to medium SAND (SM), and rock fragments Brown-red, sandy SILT (ML), trace rock fragments				4 58 9 11 10 10 10 14 14 8				•				
15.0	1051.0		Tan-red, silty medium to fine SAND (SM), trace rock fragments				7 9 7 152 10								
-	1041.0		Brown-red, sandy SILT (ML) RESIDUUM: Medium dense, red-brown, clayey fine SAND (SC)				8 12 9								
25.0 _	1036.0		Stiff to very stiff, brown-black, sandy SILT (ML)				8 14 559								
3 55.5 _			Stiff to very stiff, red-tan, sandy SILT (ML), moist	1						\prod	111/		П	\prod	\parallel
	CO	NT(Continued Next Page DUR ENGINEERING, LLC	Δ̄	= TIME	OF BO		LEGE (TOB) REMAI JTO HA	RKS	_		IOUR	REAL		∐ IG

OGGED BY: Blake				1066.0								_
DATE DRILLED: Ap	ril 12, 2014 D: Hollow Stem Auger	PTH: 70	70 ft VEL: 52.0 ft TOB WATER LEVEL: 52.0 ft									
DRILLING METHOL	. Hollow Stern Auger		RESU		EL: 52.	υ π			DATA	VEL: 5	2.0	π
DEPTH (feet) ELEVATION (feet) GRAPHIC LOG	SAMPLE DESCRIPTION	NATURAL MOISTURE CONTENT (%)	LIQUID LIMIT (%)	PLASTIC INDEX (%)	BLOW COUNTS	1	N		lue, blow	/s/ft.	50) 10
40.0	Stiff to very stiff, red-tan, sandy SILT (ML), moist (Continued)				2 4 6				•			
45.0 1021.0	Loose, white-gray-tan, silty medium to fine SAND	_			3 4 6							
50.0 1016.0 	Very stiff, brown-red-black, sandy SILT (ML), moist				3555					>		
60.0 1006.0	Medium dense, brown, silty fine SAND (SM), trace mica, wet Medium dense, brown-gray, silty fine SAND (SM), trace mica, moist				9 4 6 7				•			
65.0 1001.0	Medium dense, gray, silty fine SAND (SM), trace				7 7 5				•			
70.0 996.0	mica, moist				689							
-	Boring Terminated at 70 feet.				<u> </u>							
CONT	OUR ENGINEERING, LLC	LEGEND							EAC	NIC		

			Summers			1061.0								_
			I 11, 2014 Hollow Stem Auger	-		PTH: 70 R LEV		0 ft T/	7B 14	/ATI	ER LE	\/EI · F	<u> </u>	f+
	IO IVIL I	TIOD.	Floliow Stelli Augel		RESU		EL. 33.				ATA	VEL. 5	+ .0 i	-
DEPTH (feet) ELEVATION (feet) GRAPHIC LOG					LIQUID LIMIT (%)	PLASTIC INDEX (%)	BLOW COUNTS	1		∕alue	e, blow	/s/ft.	50) 1
5.0	1 <u>058.0</u> 1 <u>055.0</u>		FILL: Brown-red, sandy SILT (ML) Brown-red, sandy SILT (ML), trace rock fragments Red, silty medium SAND (SM), trace rock fragments				3913 6011 754			•				
10.0	1051.0		Red-white, silty coarse to medium SAND (SM), and				4 6 4							H
15.0 1	046.0		Brown-red, silty medium SAND (SM), trace rock fragments				16 20 11							
20.0 1	1041.0		Red-brown, silty fine SAND (SM), trace rock fragments				7 7 8							
25.0 <u>1</u>	1036.0		Red-brown, sandy SILT (ML), trace rock fragments, moist				533							
30.0 1	1031.0		Red-brown, sandy SILT (ML)				355							
35.0 1	1026.0		RESIDUUM: Firm, red-brown, sandy SILT (ML) Continued Next Page				4 5 7							
1	CO	NTC	OUR ENGINEERING, LLC	Ā	= TIME	OF BO		LEGEND (TOB) REMARK JTO HAM	S.		24 HC	OUR R	ΞAD	۱N

LOGGE	ED BY: E	3lake \$	Summers	ELEV	ATION	: 1061.0	O ft									1
DATE D	ORILLEI	D: Apri	l 11, 2014	BORII	NG DE	PTH: 70	0 ft									
DRILLI	NG MET	HOD:	Hollow Stem Auger		R WATE		EL: 53.	0 ft				R LE\	/EL: 5	4.0 f	<u>t</u>	_
				LAB	RESU	LTS			FI	ELD	D D	ATA				_
DEPTH (feet)	ELEVATION (feet)	GRAPHIC LOG	SAMPLE DESCRIPTION	NATURAL MOISTURE CONTENT (%)	LIQUID LIMIT (%)	PLASTIC INDEX (%)	BLOW COUNTS	1	1	N Va		, blow	s/ft.	50	10	10
			RESIDUUM: Firm, red-brown, sandy SILT (ML) (Continued)								\prod					I
40.0	1021.0						2 3 4				•					
_ _ _			Stiff, red-brown, sandy SILT (ML), trace mica, moist				2									
45.0	1016.0		Medium dense, gray-white, silty fine SAND (SM),				2 3 6					1		\coprod	\coprod	+
-	4044.0		trace mica, moist				466									
50.0	1011.0		Firm, brown, sandy SILT (ML), trace mica, moist				6									
55.0	7 1006.0						3 3 5				H					
-			Medium dense, black-brown-white, silty coarse to medium SAND (SM), moist				10 13 17									
60.00							17 12 14 20							,		
65.0 70.0 P.O. P.O. P.O. P.O. P.O. P.O. P.O. P	996.0		Dense, white-brown, silty medium to fine SAND (SM), moist				20 10 29 13									
70.0	551.0	1.1.	Boring Terminated at 70 feet.				13									
D D D D D D D D D D D D D D D D D D D	CO	NT(OUR ENGINEERING, LLC	Δ	= TIME	OF BO	ORING			Ţ	<u></u>	24 HO	UR RI	ΞAD	INC)
							A	REMA UTO H		ER			PAGE	2 ()F	,

LOGG	ED BY:	Blake	Summers	ELEV	ATION	: 1063.0) ft									
			il 11, 2014	BORI	NG DE	PTH: 1	3 ft									
DRILLI	ING ME	THOD:	Hollow Stem Auger		R WATE		EL: ft					R LE	/EL:	ft		
				LAE	RESU	LTS			FI	ELC	D D	ATA				
DEPTH (feet)	ELEVATION (feet)	GRAPHIC LOG	SAMPLE DESCRIPTION	NATURAL MOISTURE CONTENT (%)	LIQUID LIMIT (%)	PLASTIC INDEX (%)	BLOW COUNTS	1	1	N Va		, blow	s/ft.	5	0 1	00
-	1060.0		FILL: Brown, sandy SILT (ML), trace mica, trace rock fragments Brown-red, sandy SILT (ML)				8 6 6									
5.0_	1057.0		Brown-red, silty coarse to medium SAND (SM),				646 6163						7	,		
-	1052.2		trace rock fragments				9 10 8									
10.0 _	1053.0		BOULDER FILL: Sampled as gray, silty coarse SAND (SM), and rock fragments				8									
-	1050.0		Auger Refusal at 13 feet.							\mathbb{H}	\mathbb{H}	-	+	+	Н	#
45.0																
15.0 _																
25.0 _																
25.0																
2																
	CO	NT(OUR ENGINEERING, LLC	Ā	= TIME	OF BO		REMA	RKS		_ = :	24 HC	UR F	REA	DIN	IG
3							Α	ито н	AMM	ER			PAGE	= 1	OF	- 1

LOGG	ED BY: I	Blake S	Summers	ELEV	ATION:	1063.0) ft								
			l 11, 2014	BORI	NG DEI	PTH: 70	O ft								
DRILL	ING ME	THOD:	Hollow Stem Auger		WATE		EL: ft					R LEVE	_: 49.0) ft	
	£:				RESU				FI	ELC) DA	ATA			_
DEPTH (feet)	ELEVATION (feet)	GRAPHIC LOG	SAMPLE DESCRIPTION	NATURAL MOISTURE CONTENT (%)	LIQUID LIMIT (%)	PLASTIC INDEX (%)	BLOW COUNTS	1	١	N Va		, blows/ft		0 1	00
-			BORING OFFSET 20 FEET EAST TO EDGE OF SLOPE												
5.0 _			Auger only												
- 15.0 _	1050.0		FILL: Brown-red, silty medium to fine SAND (SM), trace rock fragments												
- - - 20.0_							5 10 9					•			
- - - 25.0 _	1038.0						6 7 5								
- - 30.0_	1033.0		Brown-red, sandy SILT (ML)				4 5 5								
-	1028.0		Brown-red, sandy SILT (ML), trace rock fragments				6 10 10								
35.0_	1020.0						10				$\parallel \parallel$			Ш	Ħ
<u>-</u>		. "	Continued Next Page					LEGE	:ND		Ш			Ш	Ш
	CO	NTO	OUR ENCINEERING, LLC	Ā	= TIME	OF BO		(TOB) REMA	RKS		. = 2	24 HOUF	R REA	DIN	G
							A	UTO H	MMA	ER		ΡΔι	GE 1	OF	_

LOGG	ED BY:	Blake	Summers	ELEV	ATION:	1063.0) ft								
DATE	DRILLE	D: Apr	il 11, 2014	BORII	NG DEI	PTH: 70) ft								
DRILLI	ING ME	THOD	Hollow Stem Auger	+	WATE		EL: ft					R LEV	EL: 4	9.0 f	t
	£				RESU				FI	ELC	D/	ATA			
DEPTH (feet)	ELEVATION (feet)	GRAPHIC LOG	SAMPLE DESCRIPTION	NATURAL MOISTURE CONTENT (%)	LIQUID LIMIT (%)	PLASTIC INDEX (%)	BLOW COUNTS	1	1	N Va		, blows	;/ft.	50	100
- - 40.0 _	1023.0		RESIDUUM: Medium dense, brown-red, silty fine SAND (SM) (Continued)				4 7 8					•			
-			Loose to medium dense, brown-orange, silty fine SAND (SM), moist				358					•			
45.0 _ - - - <u>-</u>	- - - - -						8 3 2 4			•					
50.0 _							2 3 5								
55.0 _ - - - - 60.0 _	1008.0		Loose, brown-white-pink, silty fine SAND (SM), wet				5 34 5								
65.0	998.0		Medium dones have all the control of				4 4 4				•				
70.0	993.0		Medium dense, brown, silty fine SAND (SM), moist				6 12 14						•		
-	-		Boring Terminated at 70 feet.												
	CO	NTO	OUR ENGINEERING, LLC	Δ	= TIME	OF BO		LEGE (TOB) REMA		Ţ	_ = 2	24 HO	JR RI	EAD'	ING
8							Al	UTO H	MMA	ER		F	AGE	2 ()F :

LOGG	ED BY:	Blake S	Summers	ELEV	ATION:	1066.0) ft								
DATE	DRILLE	D: Apri	l 10, 2014	BORII	NG DEI	PTH: 70) ft								
DRILLI	ING ME	THOD:	Hollow Stem Auger		R WATE		EL: 54.	0 ft	TOE	3 W.	ATE	ER LEV	'EL: 54	.0 ft	
				LAB	RESU	LTS			FI	IELI	D D	ATA			
DEPTH (feet)	ELEVATION (feet)	GRAPHIC LOG	SAMPLE DESCRIPTION	NATURAL MOISTURE CONTENT (%)	LIQUID LIMIT (%)	PLASTIC INDEX (%)	BLOW COUNTS	1	ı	N V:		e, blows		50	100
5.0	1056.0		FILL: Red-brown, silty medium to fine SAND (SM), trace rock fragments				688 478 668 555								
10.0 _	1051.0		Red-brown-black, sandy SILT (ML), some rock fragments				5 18 18								
20.0	1046.0		Red-brown, sandy SILT (ML) Red-brown, sandy SILT (ML), some rock fragments				4 5 8								
25.0 _	1041.0		Red-brown, silty medium to fine SAND (SM), some rock fragments				5 7 8					•			
30.0 _ -	1036.0	\(\frac{1}{2}\)	Red-brown-black, sandy SILT (ML), topsoil				5 35 24								
25.0	-	10 10 10 10 10 10 10 10 10 10 10 10 10 1	Continued Next Page				356								
	•		STATE OF THE PARTY				•	LEGI	END						ــــــــــــــــــــــــــــــــــــــ
	CO	NT(DUR ENCINEERING, LLC	Ā	= TIME	OF BO			ARKS		<u> </u>	24 HO	UR RE	ADII	√G
							Α'	ОТОП	WIALIA!			F	AGE	1 0	<u> </u>

			Summers PROJECT NO.:	ELEV	ATION:										
			I 10, 2014		NG DE			0.6	20.11	.			. 54.6		_
PKILLIN	G IVIE		Hollow Stem Auger		R WATE		⊑L: 54.				ER L DATA	EVEL	. 54.0	ı Tt	_
DEPTH (feet)	ELEVATION (feet)	GRAPHIC LOG	SAMPLE DESCRIPTION	NATURAL MOISTURE CONTENT (%)	LIQUID LIMIT (%)	PLASTIC INDEX (%)	BLOW COUNTS	1		/alu		ows/ft.		50 1	0
40.0	026.0	77.77 7.77 7.77 7.77	Red-brown-black, sandy SILT (ML), topsoil (Continued)				5 5 7				•				
	021.0		RESIDUUM: Firm, orange-brown-black, sandy SILT (ML), moist				653								
-			Stiff, orange-brown-black, sandy SILT (ML), wet				3 4 6								
- - - - ¥	016.0		Loose, tan-brown, silty fine SAND (SM), trace mica, moist	-			6 2 4 5								
60.0			Medium dense, gray-tan, silty fine SAND (SM)				11 11 11								
- - -	001.0						10 12 15						•		
-	996.0		Medium dense, gray-tan-black, silty fine SAND (SM)				10 11 11								
-	20.0	1.1.	Boring Terminated at 70 feet.				11								1
	CO	NT(OUR ENGINEERING, LLC	Ϋ́	= TIME	OF BO		REMARK	S.		= 24 H	HOUR	REA	DIN	IC
							A	JTO HAM	WEF	•		PAG	E 2	OF	=

	DRILLE	D. Anri													
						PTH: 70									_
DKILL	ING ME	THOD:	Hollow Stem Auger			R LEV	EL: 50.	0 ft				R LEV	EL: 53	3.0 ft	<u>:</u>
DEPTH (feet)	ELEVATION (feet)	GRAPHIC LOG	SAMPLE DESCRIPTION	NATURAL MOISTURE CONTENT (%) HIQUID LIMIT (%) CONTENT			BLOW COUNTS				DA	TA blows	/ft.		
				NAT	П	PL/	_ α	1		5	1	0		50	10
- - - 5.0 _	1064.0		FILL: Gray-tan, silty fine SAND (SM) Brown-red, sandy SILT (ML), trace rock fragments				555 566								
- - - 10.0 _							4 4 5 3 5 5					•			
- - - 15.0 _ -	1052.0		Brown-red, sandy SILT (ML), trace rock fragments, trace organic material				4 4 5								
20.0 _ - -	1047.0		Red-brown, sandy SILT (ML)	-			4 6 9								
25.0 _ - - - - - - - - - - - - -	1042.0		Brown, silty fine SAND (SM), moist				4 5 8								
30.0 _	1037.0		Red-brown, sandy SILT (ML), moist				655								
- - 35.0 _ -	1032.0		Continued Next Page	-			3 4 4								
	CO	NTC	OUR ENGINEERING, LLC	Ā	= TIME	OF BO		LEGE (TOB) REMA	RKS		= 2	4 HOl	JR RE	ADI	NC

LOGGED BY: Blake		ELEV	ATION:	: 1067.0								_
DATE DRILLED: Apr				PTH: 70		o (: T	00.1			F1 50		
DRILLING METHOD	: Hollow Stem Auger		RESU	R LEV	EL: 50.	0 ft I			ER LEV DATA	EL: 53	.0 ft	
DEPTH (feet) ELEVATION (feet) GRAPHIC LOG	SAMPLE DESCRIPTION	NATURAL MOISTURE CONTENT (%)	LIQUID LIMIT (%)	PLASTIC INDEX (%)	BLOW COUNTS	1	N,		e, blows	:/ft.	50	10
40.0 1027.0	RESIDUUM: Medium dense, brown-red, clayey fine SAND (SC), moist (Continued)				3 4 7							
- - - - 45.0 _	Stiff to very stiff, brown-orange, sandy SILT (ML), moist				5 7 10							
- - - -					7 7 8				•			
50.0 \$\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\					7 10 11							
55.0 1012.0	Medium dense, gray-white, silty fine SAND (SM), trace mica, moist				11 6 7				•			
60.0 1007.0	Medium dense, brown, silty fine SAND (SM), trace mica, moist				9				•			
65.0 1002.0	Dense, gray-tan, silty fine SAND (SM), trace mica, moist				7 7 10							
70.0 997.0	Boring Terminated at 70 feet.				8 16 20							
CONT	OUR ENGINEERING, LLC	Ā	= TIME	OF BO		REMAR	(S		= 24 HO	JR RE	ADI	NG
					Al	JTO HAN	IME	₹	P	AGE 2	2 0	F

LOGGI	ED BY: I	Blake S	Summers	ELEV	ATION:	1066.0) ft								_
			10, 2014	BORII	NG DEI	PTH: 70) ft								
DRILLI	NG ME	THOD:	Hollow Stem Auger		WATE		EL: 54.	0 ft					L: 53.) ft	
				LAB	RESU	LTS			FI	ELD	DAT	Ά			_
DEPTH (feet)	ELEVATION (feet)	GRAPHIC LOG	SAMPLE DESCRIPTION	NATURAL MOISTURE CONTENT (%)	LIQUID LIMIT (%)	PLASTIC INDEX (%)	BLOW COUNTS	1	Ν	I Val	ue, b 10	olows/1		50 ·	10
		\bowtie	FILL: Black-brown, sandy SILT (ML), trace rock							\prod				П	
- - - 5.0 _	1060.0		fragments, trace organic material				333 222		•						
- - -			Black-brown, sandy SILT (ML), with organic material, moist				232 333								
10.0							33								
- 15.0 _ - -	1051.0		Brown, sandy SILT (ML), trace rock fragments, trace organic material, moist				2 2 3								
20.0							232							+	
25.0 _ -	1041.0		Red-brown, silty medium to fine SAND (SM)				3 4 4								
30.0	1036.0		Brown-black, silty medium to fine SAND (SM),				3 4 4								
- - - 35.0 _	1031.0		trace organic material				4 4 6								
_			Continued New Pare								/				
			Continued Next Page					LEGE	- ND		Ш			Ш	Ц
	CO	NTC	OUR ENCINEERING, LLC	Ā	= TIME	OF BO	ORING			Ţ	= 24	HOU	R REA	/DII	10
							Al	JTO H	AMM	ER		DA	GE 1	_	_

LOGG	ED BY:	Blake	Summers	ELEV	ATION:	: 1066.0) ft							
DATE	DRILLE	D: Apri	l 10, 2014	BORI	NG DE	PTH: 70	O ft							
DRILL	ING ME	THOD:	Hollow Stem Auger		R WATE		EL: 54.	0 ft	ТОВ	WA	TER LE	VEL: 5	3.0 ft	t
				LAE	RESU	LTS			FI	ELD	DATA			
DEPTH (feet)	ELEVATION (feet)	GRAPHIC LOG	SAMPLE DESCRIPTION	NATURAL MOISTURE CONTENT (%)	LIQUID LIMIT (%)	PLASTIC INDEX (%)	BLOW COUNTS	1	N	l Valı 5	ue, blow 10	rs/ft.	50	10
			Brown, sandy SILT (ML), moist, trace organic								Ш			П
40.0	1026.0		material (Continued)				3 3 4			•				
-	-		Dark brown, sandy SILT (ML), with organic material, topsoil				22					+		
45.0 _	1021.0	7.7.7					22 10 9							
-	-		RESIDUUM: Loose, red-tan, silty fine SAND (SM), trace mica, moist											
50.0 _	- - - ¥-						2 3 4				•			
55.0 _ - -	1011.0		Loose, gray-white, silty fine SAND (SM), trace mica, moist				4 4 4							
60.0 _	1006.0		Loose, tan-orange, silty fine SAND (SM), trace mica, moist				334							
65.0_	1001.0		Dense, brown-white, silty coarse to fine SAND				3 4 5							
60.0	996.0		(SM), trace mica				10 16 15							
-	-		Boring Terminated at 70 feet.											
	CO	NTO	DUR ENCINEERING, LLC	Ā	= TIME	OF BO	DRING			Ţ	= 24 HC	UR R	EADI	ING
:			,				A	REMA UTO H		ER		PAGE	2 ()E

LOGG	ED BY: I	Blake S	Summers	1		1067.0) ft							
			l 9, 2014	BORII	NG DEI	PTH: 70) ft							
DRILL	ING ME	THOD:	Hollow Stem Auger			R LEV	EL: 46.	0 ft T			R LEVE	L: 50.0) ft	
				LAB	RESU	LTS			FIEL	D D	ATA			_
DEPTH (feet)	ELEVATION (feet)	GRAPHIC LOG	SAMPLE DESCRIPTION	NATURAL MOISTURE CONTENT (%)	LIQUID LIMIT (%)	PLASTIC INDEX (%)	BLOW COUNTS	1	N V		e, blows/		50 1	00
-			FILL: Brown-red, silty medium to fine SAND (SM), trace rock fragments				5 7 9							
5.0 _							4 6 8							<u> </u>
-	1059.0		Red-brown, silty fine SAND (SM)	_			5 7 10 7							
10.0	1057.0			_			7 8 10		Ш	Ш	\parallel / \parallel		Ш	\parallel
- - -			Red, silty medium SAND (SM), trace rock fragments				6							
15.0_	1052.0		Red, sandy SILT (ML)				6 5 4							
20.0 <u> </u>							2 3 4							
- - 25.0 _							4 8 9				•			
- - - 30.0 _							3 4 4							
-														
35.0_	1032.0			_			2 3 4			$+ \parallel \rangle$		+	\mathbf{H}	\parallel
_	-	KXXX	Continued Next Page										Ш	\coprod
	CO	NTC	DUR ENCINEERING, LLC	Δ	= TIME	OF BO		LEGENI (TOB) REMARI UTO HAN	(S		24 HOU	R REA	.DIN	G
			\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\				A	O TO HAIV	i ivi E F	`	P/	GE 1	OF	_

DATE DRILLED: April 9, 2014 DRILLING METHOD: Hollow Stem Auger	BORING DEPTH: 70 ft 24 HR WATER LEVEL: 46.0 ft LAB RESULTS FIELD DATA
	LAB RESULTS FIELD DATA
DEPTH (feet) ELEVATION (feet) GRAPHIC LOG NOITHEIGH	NATURAL MOISTURE CONTENT (%) LIQUID LIMIT (%) PLASTIC INDEX (%) BLOW COUNTS 1 2 10 20 10
Brown, silty fine SAND (SM), trace wood fragments (Continued) 40.0 RESIDUUM: Firm, red-brown, sandy SILT	5 5 8
Firm, red, sandy SILT (ML), trace mica, wet	
50.0 <u>\$\sqrt{1017.0}\$ Stiff, brown-red, sandy SILT (ML), wet \$\sqrt{55.0}\$ 1012.0</u>	3 4 7
Dense to very dense, white-tan, silty fine S/ (SM), moist	11 17 20
60.0	10 19 24 28 28 41 39
70.0 997.0 PER Boring Terminated at 70 feet.	
CONTOUR ENGINEERING, LLC	LEGEND

			Summers			: 1066.0		
			l 11, 2014			PTH: 70		
DRILL	ING ME	THOD:	Hollow Stem Auger				EL: 60.	
DEPTH (feet)	ELEVATION (feet)	GRAPHIC LOG	SAMPLE DESCRIPTION	NATURAL MOISTURE CONTENT (%)	LIQUID LIMIT (%)	PLASTIC INDEX (%)	BLOW COUNTS	N Value, blows/ft.
5.0 _	1060.0		FILL: Brown-black, sandy SILT (ML), trace rock fragments, trace organic material	Z		<u>a</u>	433	1 5 10 50 10
- - 10.0_	1056.0	1/2 1/2 1/2 1/2 1/2 1/2 1/2 1/2 1/2 1/2	Brown-black, sandy SILT (ML), trace asphalt, some organic material, moist				224 332	
-			Brown-black, sandy SILT (ML), trace rock fragments, trace organic material				3 6 12	
15.0_	1051.0		Red-brown, sandy SILT (ML), trace rock fragments, trace organic material				12 4 4 5	
20.0 _							5 457	
30.0	1036.0		Gray-brown, silty fine SAND (SM), trace mica,				346	
25.0	1031.0		moist				343	
	CO	NTC	DUR ENGINEERING, LLC	Δ	= TIME	OF BO		LEGEND G (TOB) T = 24 HOUR READING REMARKS AUTO HAMMER
								PAGE 1 OF

LOGGED				ELEV	ATION:	: 1066.0									_	_
			I 11, 2014			PTH: 70		0.4	·OD :	١٨/٨		חור	/CL . /	20.0		_
DRILLING	∍ IVI⊏ I	пор.	Hollow Stem Auger		RESU	ER LEV	EL: 60.	υπ μ	FIE			R LE\	/EL: (0.80	π	_
DEPTH (feet)	ELEVATION (feet)	GRAPHIC LOG	SAMPLE DESCRIPTION	NATURAL MOISTURE CONTENT (%)	LIQUID LIMIT (%)	PLASTIC INDEX (%)	BLOW COUNTS	1	N		ue,	, blow	s/ft.	50	0 1	0
40.0 10	026.0		Brown-red, sandy SILT (ML), trace rock fragments, moist (Continued)				3 5 7									
- - - -	021.0		RESIDUUM: Stiff, dark brown, sandy SILT (ML)				6 6 7					•				
-			Medium dense, gray-tan, silty fine SAND (SM)				356									
-	016.0		Loose, tan-red, silty fine SAND (SM), trace mica, moist				2 2 3									
- - - -	011.0		Loose, gray-white, silty fine SAND (SM), trace mica, moist				3 3 4									+
50.0 <u>\rightarrow</u> 10 - - -			Loose, brown, silty fine SAND (SM), some mica, wet				3 4 4									
_ _ _ _ _	001.0		Medium dense, red-brown, silty fine SAND (SM), trace mica, wet				5 6 8									
70.0 9	96.0		Boring Terminated at 70 feet.				8									1
	CO	NTC	OUR ENGINEERING, LLC	Δ	= TIME	OF BO		REMARI	KS		= 2	24 HC	UR F	REAL	AIC	IC
			ZHIY.	AUTO HAMMER PAGE 2										- 2	OF	=

LOGG	ED BY:	Blake S	Summers	ELEV	ATION:	: 1065.0) ft	
			l 10, 2014	BORII	NG DE	PTH: 70	O ft	
DRILL	ING ME	THOD:	Hollow Stem Auger			R LEV	EL: ft	TOB WATER LEVEL: 54.0 ft
				LAE	RESU	LTS		FIELD DATA
DEPTH (feet)	ELEVATION (feet)	GRAPHIC LOG	SAMPLE DESCRIPTION	NATURAL MOISTURE CONTENT (%)	LIQUID LIMIT (%)	PLASTIC INDEX (%)	BLOW COUNTS	N Value, blows/ft. 1 5 10 50 10
- -	1062.0		FILL: Red-brown, sandy SILT (ML), moist Red-brown, silty medium to fine SAND (SM),				0002	
5.0 _	1050.0		trace wood fragments				2 3 3	
-	1059.0		Red-brown, silty medium to fine SAND (SM), trace rock fragments				3 4 6	
10.0 _							3 4 4	
- - -	-						3 4 4	
15.0 _ - -	-							
20.0 _	-						34 5	
- - 25.0 _ -	-						3 4 6	
30.0							3 4 7	
- - -							3	
35.0 _	1030.0		Continued Next Page	_			3 4 5	
	1				ı	1	1	LEGEND
	CO	NT(DUR ENCINEERING, LLC	Δ	= TIME	OF BO	DRING	(TOB) <u>▼</u> = 24 HOUR READING REMARKS
							Al	UTO HAMMER PAGE 1 OF

LOGG	ED BY:	Blake	Summers	ELEV	ATION:	: 1065.0	O ft_									_
			il 10, 2014	BORII	NG DEI	PTH: 70	0 ft									
DRILLI	ING ME	THOD	Hollow Stem Auger	_	WATE		EL: ft					ER LE	VEL:	54.0) ft	
				LAB	RESU	LTS			F	IEL	D D	ATA				_
DEPTH (feet)	ELEVATION (feet)	GRAPHIC LOG	SAMPLE DESCRIPTION	NATURAL MOISTURE CONTENT (%)	LIQUID LIMIT (%)	PLASTIC INDEX (%)	BLOW COUNTS	1	1	N V 5		e, blo	ws/ft.	5	60 1	100
_		711/	Red-brown-black, sandy SILT (ML), trace clay,							П	Т				П	П
40.0	1025.0	<u> </u>	and organic material(Continued)				333									
-			RESIDUUM: Medium dense, orange-red-brown, silty fine SAND (SM)													
45.0 _	1020.0						5 9 10						\perp		Ш	
-			Medium dense, white-tan-gray, silty fine SAND (SM)													
50.0 _	1015.0		Medium dense, orange-brown-black, silty coarse to fine SAND (SM), some rock fragments, wet				57 6 57 7									
55.0 _	-		Medium dense, tan-orange-brown, silty fine SAND (SM), trace mica, moist				6 11 8									
60.0 _	1005.0		Medium dense, tan-orange-brown, silty medium to fine SAND (SM), moist				7 14 10						•			
65.0	995.0		Medium dense, white-tan-brown, silty fine SAND (SM), trace mica, moist				10 6 12 15									
70.0 _	333.0	P1 F. 112	Boring Terminated at 70 feet.				15								H	\parallel
-								LEGE	 END						Ш	
	_C O	NT	THE CHEST COINC II C	$ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \$	= TIME	OF BO	ORING				<u> </u>	24 H	OUR	REA	DIN	1G
	υŪ	11 11	DUR ENGINEERING, LLC				A	REMA								
				AUTO HAMMER PAGE 2 0										OF	<u></u>	

LOGGI	ED BY: I	Blake S	Gummers	ELEV	ATION:	1066.0) ft								
DATE	DRILLEI	D: Apri	l 23, 2014	BORII	NG DEI	PTH: 7	1 ft								
DRILLI	NG ME	THOD:	Hollow Stem Auger		R WATE		EL: ft		_			ER LEVE	L: 50.	0 ft	
				LAE	RESU	LTS			F	IEL	D D	ATA			
DEPTH (feet)	ELEVATION (feet)	GRAPHIC LOG	SAMPLE DESCRIPTION	NATURAL MOISTURE CONTENT (%)	LIQUID LIMIT (%)	PLASTIC INDEX (%)	BLOW COUNTS	1		N V 5		e, blows/ 10		50	10
			FILL: Red-brown, sandy SILT (ML), trace rock												П
5.0			fragments				459 566								
- - -							5 7 4								
10.0 _	1056.0	\bowtie					4 6 7			Ш	Ш	$\parallel \downarrow \downarrow$	$\perp \downarrow \downarrow$	Ц	Ш
- - -	1051.0		Red-brown, sandy SILT (ML), and rock fragments				8 9 11								Í
15.0 _ - - - -			Red-brown, sandy SILT (ML), trace rock fragments				5 6 7								
20.0 _	1046.0		Red-brown, sandy SILT (ML)				7 3 7 7					•			
25.0							337								
30.0							7 4 7 9								
35.0 _	1031.0			1			9			+	H	++	++	+	\mathbb{H}
_			Continued Next Page											Ш	Ш
	CO	NTO	OUR ENGINEERING, LLC	Σ	= TIME	OF BO		REMA	RKS			24 HOU	R RE	ADII	۷G
							A	ито н	AMM	IER			AGE 1	_	_

LOGG	ED BY:	Blake \$	Summers	ELEV	ATION	1066.0	0 ft							
			I 23, 2014	BORI	NG DE	PTH: 7	1 ft							
DRILL	ING ME	THOD:	Hollow Stem Auger	_	R WATE		EL: ft				ER LEV	/EL: 50	.0 ft	
	et)	(D			RESU				FIEI	_D D	ATA		—	
DEPTH (feet)	ELEVATION (feet)	GRAPHIC LOG	SAMPLE DESCRIPTION	NATURAL MOISTURE CONTENT (%)	LIQUID LIMIT (%)	PLASTIC INDEX (%)	BLOW COUNTS	1			e, blows	s/ft.	50	100
40.0 _	1026.0		RESIDUUM: Stiff, dark brown-red, sandy SILT (ML) (Continued) Loose, red, silty medium to fine SAND (SM)				359				•			
- - - 45.0 _	1021.0		Loose, red, sitty medium to line SAND (SM)				655							
-	-		Firm, tan-brown, sandy SILT (ML), moist				233							
- - -	1016.0		Medium dense, brown-white, silty medium SAND (SM), moist				388				•			
55.0 _	1011.0		Very stiff, brown, sandy SILT (ML), trace mica, moist											
60.0 _							3 8 14							
65.0 _	1001.0		PARTIALLY WEATHERED ROCKSampled as white-tan, silty medium SAND (SM)				3 11 20						$\frac{1}{1}$	
65.0 _	005.5						50/1"							
_	995.0	K	Auger Refusal at 71 feet.											
	CO	NT(OUR ENGINEERING, LLC	Ā	= TIME	OF BO	ORING	LEGEN (TOB) REMAR		▼ =	24 HO	UR RE	ADII	٧G
				AUTO HAMMER PAGE 2 OF									= ;	

LOGG	ED BY:	Blake S	Summers	ELEV	ATION:	: 1065.0	O ft								_
DATE	DRILLE	D: Apri	l 22, 2014	BORII	NG DE	PTH: 70	6 ft								
DRILLI	ING ME	THOD:	Hollow Stem Auger		WATE		EL: ft					ER LEV	EL: 56	.0 ft	_
					RESU	LTS		1	FI	IELI	D D	ATA			_
DEPTH (feet)	ELEVATION (feet)	GRAPHIC LOG	SAMPLE DESCRIPTION	NATURAL MOISTURE CONTENT (%)	LIQUID LIMIT (%)	PLASTIC INDEX (%)	BLOW COUNTS	1	1	N V		e, blows		50	10
			FILL: Brown, silty fine SAND (SM)				_								П
- - 5.0 _	1062.0		Red-brown, sandy SILT (ML), trace rock fragments	-			345 445 5						P		
- - 10.0_	1055.0		Brown-red, sandy SILT (ML), moist	-			5 10 12 5 7 9					•			
- - - 15.0 _ - -			Diowit fou, Sarray OILT (IVIL), HIGIST				225				•				
20.0 _ - - -							4 4 6								
25.0 _ - - -	1040.0		Gray-brown, sandy SILT (ML), trace mica, moist				546 366								
30.0_	1035.0		Brown-red, sandy SILT (ML), trace rock fragments, moist				347								
35.0 _	1030.0			1			'		\top	\dagger	$\dagger\dagger$			$\dagger\dagger$	\dagger
	1		Continued Next Page					1505			Ш	11'	Ш	Ш	Ц
	CO	NTO	OUR ENCINEERING, LLC	Δ	= TIME	OF BO		REMA	RKS		<u></u> =	24 HOL	JR RE	ADII	NG
				AUTO HAMMER PAGE 1											F

			Summers PROJECT NO.:	ELEV	ATION:	: 1065.0								_
			I 22, 2014 Hollow Stem Auger	+		PTH: 70		TOF		TED		. 50.0		
DKILLI	ING ME	HOD	Hollow Stern Auger		RESU	R LEV	EL: ft			DAT	LEVEL _A	.: 56.0) ft	_
DEPTH (feet)	ELEVATION (feet)	GRAPHIC LOG	SAMPLE DESCRIPTION	NATURAL MOISTURE CONTENT (%)	LIQUID LIMIT (%)	PLASTIC INDEX (%)	BLOW COUNTS				olows/ft.		50 1	_
40.0	1025.0		RESIDUUM: Stiff, brown-red, sandy SILT (ML), trace clay (Continued)				455			•				
- - -	1020.0		Loose, brown-red, silty fine SAND (SM)				5 5 4			•				
- - -			Loose, brown-white, silty medium to fine SAND (SM), wet				4 3 4							
55.0	1015.0		Stiff, tan, sandy SILT (ML), wet				5 5 7							
- - - Z	<u>7</u> 1005.0						5 87							
- - - - 65.0 _			Medium dense, tan, silty fine SAND (SM), trace mica, moist				4 8 16							
70.0	995.0						4 7 11							
- -			PARTIALLY WEATHERED ROCK Dark gray, silty medium to fine SAND (SM), trace rock fragments Continued Next Page					150515						
	CO	NT(OUR ENGINEERING, LLC	Ā	= TIME	OF BO	ORING	REMARKS		= 24	HOUR	REA	DIN	10
							Al	UTO HAMM	ER		ΡΔΩ	GE 2	01	=

LOGGED BY	: Blake	Summers	ELEV	ATION:	: 1065.	0 ft								_	
DATE DRILL			BORII	NG DE	PTH: 7	6 ft									
DRILLING M	IETHOD	: Hollow Stem Auger		WATE		EL: ft		_			ER LE	VEL: 5	6.0	ft	_
			LAB	RESU	LTS			I	FIE	LD [DATA				
DEPTH (feet) ELEVATION (feet)	GRAPHIC LOG	SAMPLE DESCRIPTION	NATURAL MOISTURE CONTENT (%)	LIQUID LIMIT (%)	PLASTIC INDEX (%)	BLOW COUNTS	1			Valu 5	ie, blow 10	/s/ft.	50	0 1	(
		PARTIALLY WEATHERED ROCKDark gray,					'		T	ÍП	П	\top	TÏ	N	
75.0 _ 989.		PARTIALLY WEATHERED ROCK Dark gray, silty medium to fine SAND (SM), trace rock fragments (Continued)				50/1"								 	_
303.	0 , 11, 11	Auger Refusal at 76 feet.												\forall	
80.0															
														П	
-															
85.0															
														П	
90.0													+	+	
- - - -															
95.0								\vdash	+	\mathbb{H}	H	++	+	\dashv	ŀ
- - - -															
100.0								\vdash	+	\mathbb{H}			+	+	ŀ
- - - -															
105.0 <u> </u>														$\frac{1}{1}$	
C	ONTO	DUR ENGINEERING, LLC	Δ	= TIME	OF BO	ORING	(TOB)			▼ =	= 24 HC	OUR R	EAD	AIC	Į
						A	ито н			₹					

LOGG	ED BY:	Blake	Summers	ELEV	ATION:	: 1065.0) ft								
			il 12, 2014	BORII	NG DEI	PTH: 70	O ft								
DRILL	ING ME	THOD	Hollow Stem Auger		R WATE		EL: ft					R LEV	EL: ft		
				LAB	RESU	LTS			FI	IELI	D D	ATA			
DEPTH (feet)	ELEVATION (feet)	GRAPHIC LOG	SAMPLE DESCRIPTION	NATURAL MOISTURE CONTENT (%)	LIQUID LIMIT (%)	PLASTIC INDEX (%)	BLOW COUNTS	1	1	N V:		e, blows		50	10(
			FILL: Red-brown, sandy SILT (ML), trace rock fragments				_								
-	1062.0		Red-brown, silty medium to fine SAND (SM), trace rock fragments	-			656 611 12						•		
5.0_	1059.0						12						\square	\dagger	Н
-	1000.0		Red-brown, clayey fine SAND (SC), trace rock fragments	-			7 10 17								
_							10 15 15						🛉		
10.0 _	1055.0		Red-brown, sandy SILT (ML)	-			15			\vdash	\mathbb{H}	\vdash	/	+	₩
-	1050.0						469								
15.0 _	1030.0		RESIDUUM: Stiff, purple-brown-black, sandy SILT (ML)									•			
20.0_	1045.0		Firm, orange-brown-black, sandy SILT (ML), moist	_			4 7 8								
- - 25.0 <u>-</u>	1040.0						3 3 4				$ \mathbb{N}$				
-			Medium dense, pink-red, silty fine SAND (SM)				4 6 8								
30.0_	1035.0		Stiff, purple-brown, sandy SILT (ML), moist				8								
35.0_	_						4 5 10							$\frac{\parallel}{\parallel}$	
	1	' ' '	Continued Next Page					LEGE	: NID	Ш	Ш	П.		Ш	Ш
	CO	NT(OUR ENGINEERING, LLC	Ā	= TIME	OF BO	DRING				<u>_</u> =	24 HOI	JR RE	1DI	١G
3							A	JTO H				P	AGE 1	0	F :

	e Summers			1065.0						_
DATE DRILLED: A				PTH: 70			 	-, -		_
DRILLING METHO	D: Hollow Stem Auger			R LEV	EL: ft		ER LEVI	EL: ft		_
DEPTH (feet) ELEVATION (feet) GRAPHICLOG	SAMPLE DESCRIPTION	NATURAL MOISTURE CONTENT (%)	LIQUID LIMIT (%)	PLASTIC INDEX (%)	BLOW COUNTS	N	e, blows		50	10
40.0	Stiff, purple-brown, sandy SILT (ML), moist (Continued)				456		•			
45.0 1020.0	Stiff to very stiff, orange-brown-black, sandy SILT (ML), moist				356		•			
50.0					3455					
55.0 _					4 6 6					
60.0 1005.0	Very stiff, white-pink-brown, sandy SILT (ML)				7 8 13					
65.0 1000.0	Very stiff, orange-brown-black, sandy SILT (ML)				8 10 15			•		
70.0 995.0	(112)				7 7 9					
70.0 995.0	Boring Terminated at 70 feet.				9					
CON	OUR ENGINEERING, LLC	<u>Ā</u>	= TIME	OF BC		LEGEND (TOB) REMARKS JTO HAMME	24 HOL	JR RE	ADI	NC

FILL: Red-brown, sliv coarse to medium SAND SM, with rock fragments Red-brown, sandy SiLT (ML), trace rock fragments Red-brown, sandy SiLT (ML), trace rock fragments RESIDUMM: Stiff to very stiff, brown-red, sandy SILT (ML), trace clay RESIDUMM: Stiff to very stiff, brown-red, sandy SILT (ML), trace clay RESIDUMM: Stiff to very stiff, brown-red, sandy SILT (ML), trace mica Total Continued Next Page CONTOUR ENGINEERING, LLC REMARKS AUTO HAMMER Total Continued Next Page LEGEND Total Continued Next Page	LOGGED BY: Blake Summers		ATION:							
LAB RESULTS FIELD DATA		BORII	NG DE	PTH: 12	27 ft					
SAMPLE DESCRIPTION SAMPLE DESCRIPTION SAMPLE DESCRIPTION SAMPLE DESCRIPTION SAMPLE DESCRIPTION SAMPLE DESCRIPTION SAMPLE DESCRIPTION SAMPLE DESCRIPTION SAMPLE DESCRIPTION N Value, blows/II. N Value, blows/II. SAMPLE DESCRIPTION N Value, blows/II. SAMPLE DESCRIPTION N Value, blows/II. SAMPLE DESCRIPTION SAMPLE DESCRIPTION SAMPLE DESCRIPTION SAMPLE DESCRIPTION N Value, blows/II. N Value, blows/II. SAMPLE DESCRIPTION SAMPLE D	DRILLING METHOD: Hollow Stem Auger				EL: ft					
1063.0 Filt. Red-brown, silty coarse to medium SAND		LAB	RESU	LTS		FIELD DATA				
1063.0 Red-brown, sandy SiLT (ML), trace rock fragments 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.	DEPTH (feet) ELEVATION (feet) GRAPHIC LOG MOITGINGS AND	NATURAL MOISTURE CONTENT (%)	LIQUID LIMIT (%)	PLASTIC INDEX (%)	BLOW COUNTS					
10.0 1051.0 RESIDUUM: Stiff to very stiff, brown-red, sandy SiLT (ML), trace clay 20.0 1041.0 Firm, brown, sandy SiLT (ML), trace mica 25.0 1041.0 Firm, brown, sandy SiLT (ML), trace mica 30.0 1036.0 Medium dense, tan-red, silly fine SAND (SM) Continued Next Page CONTOUR ENSINEERING, LLC □ □ □ □ □ □ □ □ □ □ □ □ □ □ □ □ □ □ □	(SM), with rock fragments	-								
10.0 1051.0 RESIDUUM: Stiff to very stiff, brown-red, sandy SiLT (ML), trace clay 20.0 1041.0 Firm, brown, sandy SiLT (ML), trace mica 50 1031.0 Firm, brown, sandy SiLT (ML), trace mica Continued Next Page CONTOUR ENGINEERING, LLC REMARKS AUTO HAMMER	5.0				56					
20.0 1041.0 Firm, brown, sandy SILT (ML), trace mica 25.0 1041.0 Firm, brown, sandy SILT (ML), trace mica 30.0 1036.0 Medium dense, tan-red, silty fine SAND (SM) Continued Next Page CONTOUR ENGINEERING, LLC RESIDUM: Stiff to very stiff, brown-red, sandy 4 4 6 9 4 4 6 9 25.0 1041.0 LEGEND ▼ = TIME OF BORING (TOB) ▼ = 24 HOUR READING REMARKS AUTO HAMMER					3 6 7	$ \cdot \cdot$				
20.0 1041.0 Firm, brown, sandy SiLT (ML), trace mica 25.0 1041.0 Firm, brown, sandy SiLT (ML), trace mica 3 4 4 6 8 9 1036.0 1031.0 Continued Next Page CONTOUR ENGINEERING, LLC RESIDUMM: Stiff to very stiff, brown-red, sandy 4 4 6 9 10 10 10 10 10 10 10 10 10 10 10 10 10					4 6 7					
RESIDUM; Stiff to very stiff, brown-red, sandy 20.0										
25.0 1041.0 Firm, brown, sandy SILT (ML), trace mica 30.0 1036.0 Medium dense, tan-red, silty fine SAND (SM) Continued Next Page CONTOUR ENGINEERING, LLC EGEND ☐ □ □ □ □ □ □ □ □ □ □ □ □ □ □ □ □ □ □	RESIDUM: Stiff to very stiff, brown-red, sandy SILT (ML), trace clay									
Firm, brown, sandy SILT (ML), trace mica 30.0 1036.0 Medium dense, tan-red, silty fine SAND (SM) 35.0 1031.0 Continued Next Page CONTOUR ENGINEERING, LLC □ □ □ □ □ □ □ □ □ □ □ □ □ □ □ □ □ □					5					
CONTOUR ENGINEERING, LLC Medium dense, tan-red, silty fine SAND (SM) Continued Next Page LEGEND ▼ = TIME OF BORING (TOB) REMARKS AUTO HAMMER	Firm, brown, sandy SILT (ML), trace mica				3 4 4					
CONTOUR ENGINEERING, LLC CONTOUR ENGINEERING, LLC	Medium dense, tan-red, silty fine SAND (SM)									
CONTOUR ENCINEERING, LLC	Continued Next Page									
PAGE 1 OF	CONTOUR ENGINEERING, LLC	▼ = TIME OF BORING (TOB) REMARKS AUTO HAMMER								

Loose, brown, silty coarse to medium SAND (SM), trace rock fragments (Continued) 40.0 1026.0 Firm, brown-red, sandy SiLT (ML), moist 50.0 V1016.0 Loose to medium dense, brown, silty medium to fine SAND (SM), wet 50.0 V1016.0 Very stiff, brown-white, sandy SiLT (ML), moist 50.0 V1016.0 Loose to medium dense, brown, silty medium to fine SAND (SM), wet 55.0 1001.0 Very stiff, brown-white, sandy SiLT (ML), moist 65.0 1001.0 Very stiff to stiff, brown, sandy SiLT (ML), trace 70.0 Continued Next Page	LOGGED BY: Blake	Summers PROJECT NO.: (ELEV	ATION:									
LAB RESULTS FIELD DATA													_
SAMPLE DESCRIPTION N Value, blows/ft. Sample DESCRIPTION N Value, blows/ft. A 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4	DRILLING METHOD	: Hollow Stem Auger				EL: ft					_: 50.0) ft	_
trace rock fragments (Continued) 40.0 1026.0 Firm, brown-red, sandy SILT (ML), moist 45.0 1021.0 Firm, brown-purple-white, sandy SILT (ML), moist 50.0 1021.0 Loose to medium dense, brown, silty medium to fine SAND (SM), wet 50.0 1006.0 Very stiff, brown-white, sandy SILT (ML), moist 4 8 8 8 8 8 8 9 9 9 9 9 9 9 9 9 9 9 9 9	DEPTH (feet) ELEVATION (feet) GRAPHIC LOG					BLOW COUNTS	1	N Va	alue, l	blows/ft		50 1	100
Firm, brown-red, sandy SiLT (ML), moist 1021.0 Firm, brown-purple-white, sandy SiLT (ML), moist 50.0 V1016.0 Loose to medium dense, brown, slity medium to fine SAND (SM), wet 55.0 60.0 Very stiff, brown-white, sandy SiLT (ML), moist 70.0 Continued Next Page	40.0 1026.0	trace rock fragments (Continued)				3 4 4							
Firm, brown-purple-white, sandy SILT (ML), moist 50.0 1006.0 Loose to medium dense, brown, silty medium to fine SAND (SM), wet 55.0 1006.0 Very stiff, brown-white, sandy SILT (ML), moist 65.0 1001.0 Very stiff to stiff, brown, sandy SILT (ML), trace mica, moist 70.0 Continued Next Page	- - -	Firm, brown-red, sandy SILT (ML), moist							•				
Loose to medium dense, brown, silty medium to fine SAND (SM), wet 55.0	- - - -	Firm, brown-purple-white, sandy SILT (ML), moist											
65.0 1001.0 Very stiff, brown-white, sandy SILT (ML), moist Very stiff to stiff, brown, sandy SILT (ML), trace mica, moist Continued Next Page		Loose to medium dense, brown, silty medium to fine SAND (SM), wet											
Very stiff to stiff, brown, sandy SILT (ML), trace mica, moist 70.0	60.0 1006.0	Very stiff, brown-white, sandy SILT (ML), moist											
70.0	65.0 1001.0					4 8 12							
	70.0					4 6 12							
LEGEND		Continued Next Page								1			Ш
CONTOUR ENGINEERING, LLC	CONT		Ā	= TIME	OF BO		REMARK	<u>▼</u> S	= 24	1 HOUF	≀ REA	<u>.DIN</u>	10

LOGGI	ED BY: E	Blake :	Summers	ELEV	ATION:	1066.0) ft								
DATE	DRILLEI	D: Apr	il 22, 2014	BORII	NG DEI	PTH: 12	27 ft								
DRILLI	ING MET	THOD	Hollow Stem Auger	24 HF	WATE	R LEV	EL: ft		TOE	3 W	ΆΤΕ	ER LEVE	EL: 50.	0 ft	
				LAE	RESU	LTS			F	IEL	D D	ATA			
DEPTH (feet)	ELEVATION (feet)	GRAPHIC LOG	SAMPLE DESCRIPTION	NATURAL MOISTURE CONTENT (%)	LIQUID LIMIT (%)	PLASTIC INDEX (%)	BLOW COUNTS	1	ļ	N V 5		e, blows/		50 <i>′</i>	100
- 75.0 _			Very stiff to stiff, brown, sandy SILT (ML), trace mica, moist (Continued)				4 5 8								
75.0							o 350					•			
-							4 5 8					•			
85.0	981.0		Stiff, tan-white-red, sandy SILT (ML), trace mica, moist				98 456					•			
90.0	970.0		Very stiff, brown, sandy SILT (ML), trace mica, moist				3 7 10					•			
							4 6 9					•			
100.0							4 6 10					•			
			Continued Next Page										·		
					T18.47	OF 50	סיאים	LEGE	-ND	_	•	04 1101	ום פר	۰	VIC.
	CO	NT(DUR ENCINEERING, LLC	Ţ	= TIME	OF BC		(TOB) REMA UTO H				24 HOL	K KEA	ווטי	NG
												P	AGE 3	OI	= _

LOGG	ED BY: I	Blake	Summers	ELEV	ATION:	: 1066.0	O ft									٦
DATE	DRILLEI	D: Apr	il 22, 2014	BORI	NG DE	PTH: 12	27 ft									٦
DRILLI	ING ME	THOD	: Hollow Stem Auger	24 HF	R WATE	R LEV	EL: ft		TOI	3 W	ATE	R LE	VEL:	50.0	ft	
				LAE	RESU	LTS			F	IEL	D D	ATA				
DEPTH (feet)	ELEVATION (feet)	GRAPHIC LOG	SAMPLE DESCRIPTION	NATURAL MOISTURE CONTENT (%)	LIQUID LIMIT (%)	PLASTIC INDEX (%)	BLOW COUNTS	1		N V 5		e, blow	/s/ft.	5(0 10	00
110.0 _	956.0		Very stiff, brown, sandy SILT (ML), trace mica, moist (Continued) Hard, brown, sandy SILT (ML), trace rock fragments, moist				6 10 12									
- - 115.0 _	951.0		Very stiff, brown-black, sandy SILT (ML), moist				10 15 20							>		
- - - 120.0 _	-						4 7 11									
-							8 11 17									
125.0 _	939.0		Auger Refusal at 127 feet.				17								#	
130.0 _									-						+	
135.0																
	-															
1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000	-															
ECH BORING	CO	NT(DUR ENGINEERING, LLC	Ā	= TIME	OF BO	DRING	LEGE (TOB)			<u> </u>	24 HC	DUR F	REAL	NIC	G
<u> </u>							Α	ито н					PAGE	- 4	<u>OF</u>	

LOGG	ED BY: I	Blake S	Summers		ATION:	1064.0) ft								
			l 12, 2014	BORII	NG DEI	PTH: 70) ft								
DRILL	ING ME	THOD:	Hollow Stem Auger	24 HR	WATE	R LEV	EL: 47.	0 ft	TOE	3 WA	ATE	R LEV	EL: 47	.0 ft	
				LAB	RESU	LTS			FI	ELC	DA C	TA			
DEPTH (feet)	ELEVATION (feet)	GRAPHIC LOG	SAMPLE DESCRIPTION	NATURAL MOISTURE CONTENT (%)	LIQUID LIMIT (%)	PLASTIC INDEX (%)	BLOW COUNTS	1	١	N Va		blows 0		50	10(
			FILL: Red-brown, silty medium to fine SAND (SM), some rock fragments												
-	1061.0		Tan-brown, silty medium to fine SAND (SM), trace rock fragments				999 569								
5.0_	1058.0						9				$\parallel \parallel$				Ш
-	1030.0		Red-brown, silty medium to fine SAND (SM), trace rock fragments				4 4 4								
_	-	\bowtie					2 3 6								
10.0 _	1						ĕ					<u> </u>			Н
- - - 15.0 _							5 7 8								
-							459					•			
20.0 _							569					•			
25.0	1039.0		White-tan-brown, silty coarse to fine SAND (SM), and rock fragments												
30.0 _	1034.0		Red-brown, silty medium to fine SAND (SM), some rock fragments				6 4 35 35								
35.0_	1029.0						3 4 6			Ш	Щ	\		$\perp \mid$	Ш
			Operations of Maria 2									\		\prod	
			Continued Next Page					LEGE	-ND	Ш	Ш				Ш
	CO	NTO	DUR ENGINEERING, LLC	Ā	= TIME	OF BO			RKS		_ = 2	4 HOU	JR RE	ADII	١G
			VIII V				A	O I O A	MIVI IVI	Ľĸ		D	AGE 1		_

			Summers	ELEV	ATION:	: 1064.0			
			il 12, 2014			PTH: 70		- 0.00 TOD WATER LEVEL 47.0	
DRILLIN	NG ME	HOD	: Hollow Stem Auger		RESU	R LEV	EL: 47.	.0 ft TOB WATER LEVEL: 47.0 FIELD DATA	ft
DEPTH (feet)	ELEVATION (feet)	GRAPHIC LOG	SAMPLE DESCRIPTION	NATURAL MOISTURE CONTENT (%)	LIQUID LIMIT (%)	PLASTIC INDEX (%)	BLOW COUNTS	N Value, blows/ft.	0 10
40.0	1024.0		RESIDUUM: Firm to stiff, tan-orange-brown, sandy SILT (ML) (Continued)				4 7 7	•	
- - -			Firm, tan-orange-brown, sandy SILT (ML), moist				3 4 4	•	
- ¥	1019.0		Firm, orange-purple-brown, sandy SILT (ML), some rock fragments, wet				233		
- - -	1014.0		Firm to stiff, red-brown-black, sandy SILT (ML), trace mica, moist				233	•	
55.0							4 4		
60.0	999.0						4 4 5 6		
- - -	994.0		Stiff, orange-purple-brown, sandy SILT (ML), trace rock fragments, trace mica, moist				ზ ფონ		
70.0	<i>33</i> 4.∪		Boring Terminated at 70 feet.				ь		
	CO	NT(OUR ENGINEERING, LLC	Ā	= TIME	OF BO		REMARKS	DING
							Al	UTO HAMMER PAGE 2	OF

LOGG	ED BY:	Blake	Summers		ATION:	: 1058.0	O ft								_
			il 11, 2014	BORII	NG DE	PTH: 7	0 ft								
DRILL	ING ME	THOD:	Hollow Stem Auger	24 HR	WATE	R LEV	EL: ft		TOB	WAT	ER LE	VEL:	ft		
				LAB	RESU	LTS			FIE	LD [DATA				
DEPTH (feet)	ELEVATION (feet)	GRAPHIC LOG	SAMPLE DESCRIPTION	NATURAL MOISTURE CONTENT (%)	LIQUID LIMIT (%)	PLASTIC INDEX (%)	BLOW COUNTS	1		Valu 5	ie, blov	ws/ft.	50) 1	00
_			FILL: Red-brown, sandy SILT (ML), trace organic material												
_	_		material				8 10 8				/	7			
-	1055.0		Red-brown, silty medium to fine SAND (SM),	-			0				$\parallel \parallel \perp$				
-			trace organic material				8 7 6				🕇				
5.0_							6				++-		+	$^{+}$	H
-	1052.0		Brown, sandy SILT (ML), trace mica	_			6				•				
-	1050.0						6 5 6								
-	1030.0		Brown-red, sandy SILT (ML)	1			6								
10.0_	1048.0						6 5 5								
10.0			Brown-red, silty coarse to medium SAND (SM), trace rock fragments]							$\prod \setminus$			П	П
_			trace rock fragments								$ \setminus$				
_											$ \setminus$				
_	_						8 9 6				📍	•			
15.0 _	1043.0		Brown, sandy SILT (ML)	-			ĕ				\mathbb{H}		+	$^{+}$	$^{\parallel}$
-			Blown, Sandy Sier (ME)								/				
-		\bowtie									/				
-															
20.0	1038.0						5 5								
20.0_	1030.0		Brown-red, silty coarse to medium SAND (SM),	1			5				$\mathbb{H} \setminus$		+	$\dagger \dagger$	Ħ
-			trace rock fragments								$ \setminus$				
		\bowtie										$\setminus \mid$			
							18								
25.0 _	1033.0		 				18 12 10			Ш	/	44	$\perp \! \! \perp$	Ц	\parallel
-		\bowtie	Red, sandy SILT (ML), moist												
-											XII				
-	-									И					
	1						2 2 3								
30.0 _	1						3		+	Ш	Ш		+	+	H
-	1														
-															
							2			igwedge					
35.0_	1						2 3 2			Ш	Щ	$\perp \perp \perp$	$\perp \! \! \perp$	Ц	Ц
_		\bowtie	Continued Next Bene							$ \rangle$					
		1	Continued Next Page				<u> </u>	LEGEN	ND ND		Ш			Ш	Ш
	CO	NIT	DUR ENGINEERING, LLC	∑	= TIME	OF BO	ORING			▼ =	= 24 H(OUR R	EAC	NIC	G
25.0	υÜ	IV I C	JUN CHURICOMNU, LLU					REMAR							
							A	JTO HA	MME	R		D40-			_
												PAGE	. 1 (υF	2

			Summers	ELEV	ATION	: 1058.0			
			il 11, 2014			PTH: 70		TOD WATER LEVEL "	
DRILLI	NG ME	THOD	Hollow Stem Auger	_	R WATE RESU	ER LEV	EL: ft	TOB WATER LEVEL: ft FIELD DATA	
DEPTH (feet)	ELEVATION (feet)	GRAPHIC LOG	SAMPLE DESCRIPTION	NATURAL MOISTURE CONTENT (%)	LIQUID LIMIT (%)	PLASTIC INDEX (%)	BLOW COUNTS	N Value, blows/ft.) 10
- - 40.0	1018.0		Red, sandy SILT (ML), moist (Continued) RESIDUUM: Stiff, tan-brown, sandy SILT (ML), moist				555		
- - 45.0 _	1013.0		Firm, brown-orange, sandy SILT (ML), moist	_			685		
- - 50.0 _ -	1008.0		Firm to stiff, brown-purple, sandy SILT (ML), trace mica, moist				233		
- - 55.0 _							2 3 3		
- - 60.0							3 3 4		
- - 65.0 _							3 4 5		
- - - 70.0 _	988.0						4 6 7		
-			Boring Terminated at 70 feet.						
	CO	NT	DUR ENGINEERING, LLC	Δ	= TIME	OF BO		LEGEND G (TOB) T = 24 HOUR READ REMARKS AUTO HAMMER PAGE 2	

LOGG	ED BY:	Blake \$	Summers	ELEV	ATION:	: 1060.0) ft						
			1 22, 2014	BORII	NG DE	PTH: 9:	2 ft						
DRILL	ING ME	THOD:	Hollow Stem Auger	_		R LEV	EL: ft			TER LE	VEL:	55.0 f	<u>t</u>
				LAB	RESU	ILTS		F	IELD I	DATA			
DEPTH (feet)	ELEVATION (feet)	GRAPHIC LOG	SAMPLE DESCRIPTION	NATURAL MOISTURE CONTENT (%)	LIQUID LIMIT (%)	PLASTIC INDEX (%)	BLOW COUNTS	1	N Valu	ue, blov	ws/ft.	50	100
5.0_			FILL: Red-brown, sandy SILT (ML), trace rock fragments				3 4 4 3 4 6						
-	1054.0		Brown-black, sandy SILT (ML), and organic material	_			3 3 4						
10.0_	1052.0	<u>1/ 1//</u> 1	Red-black, sandy SILT (ML), some organic material Red-brown, silty coarse SAND (SM), with rock	-			3 5 10						
-			fragments				7 14 13						
15.0_	1045.0		Brown-white, silty coarse SAND (SM), with rock fragments	_			13 4 23 17						
20.0_	1040.0		Brown, sandy SILT (ML), some rock fragments				17 2 3 5						
25.0	1035.0		Red-brown, sandy SILT (ML)										
30.0 _	1030.0		Red-brown, sandy SILT (ML), moist				433						
35.0	1025.0		Continued Next Page	-			3 4 4						
	CO	NT(DUR ENCINEERING, LLC	Ā	= TIME	OF BO	ORING	LEGEND (TOB) REMARKS		= 24 H	OUR R	EAD	ING
							Al	UTO HAMM			PAGE	1 C)F (

LOGG	ED BY: I	Blake \$	Summers	ELEV	ATION	: 1060.0	O ft	
			I 22, 2014	BORI	NG DE	PTH: 9	2 ft	
DRILL	ING ME	THOD:	Hollow Stem Auger		R WATE		EL: ft	
				LAE	RESU	ILTS		FIELD DATA
DEPTH (feet)	ELEVATION (feet)	GRAPHIC LOG	SAMPLE DESCRIPTION	NATURAL MOISTURE CONTENT (%)	LIQUID LIMIT (%)	PLASTIC INDEX (%)	BLOW COUNTS	N Value, blows/ft.
40.0	1020.0		RESIDUUM; Stiff, red-brown, sandy SILT (ML), moist (Continued)				866	
- - - 45.0 _	-		Stiff, tan-black, sandy SILT (ML)				4 6 6	•
50.0 _	1010.0						7 5 8	
-	√1005.0		Loose, tan-white, silty fine SAND (SM), moist				4 4 6	
-	-		Medium dense, tan-black, silty fine SAND (SM), moist				4 6 9	•
60.0	1000.0		Loose, tan-white, silty fine SAND (SM), wet				9 346	
65.0	995.0		Medium dense, tan-black, silty fine SAND (SM), wet	_			7 8 12	
70.0 _	990.0		Very stiff to hard, gray-brown, sandy SILT (ML), trace mica, moist Continued Next Page	_			12	LECEND
	CO	NT(DUR ENGINEERING, LLC	$\overline{\Delta}$	= TIME	OF BO		LEGEND G (TOB) T = 24 HOUR READING REMARKS AUTO HAMMER
5L			THE				A	PAGE 2 OF

			PROJECT NO.:	ELEV	ATION:										_
			22, 2014	_	NG DE			T_	05:			-,			_
DRILLI	ING ME	THOD:	Hollow Stem Auger	_	R WATE RESU		EL: ft	1			TER L DATA	EVEL:	55.0) ft	_
DEPTH (feet)	ELEVATION (feet)	GRAPHIC LOG	SAMPLE DESCRIPTION	NATURAL MOISTURE CONTENT (%)	LIQUID LIMIT (%)	PLASTIC INDEX (%)	BLOW COUNTS	1	N			ows/ft.	5	50 1	_
- - 75.0 _			Very stiff to hard, gray-brown, sandy SILT (ML), trace mica, moist (Continued)				7 8 15								
80.0							12 15 22								+
- - 85.0 _	975.0		PARTIALLY WEATHERED ROCKSampled as gray, silty fine SAND (SM), trace mica				13 17 20								_
90.0 _	968.0						50/2"								
95.0_	900.0		Auger Refusal at 92 feet.												
- - - _ 100.0															
- - -															
- - - -															
	CO	NTC	OUR ENGINEERING, LLC	Ā	= TIME	OF BO	ORING	REMAR	<s< td=""><td></td><td>= 24 H</td><td>HOUR</td><td>REA</td><td>DIN</td><td>10</td></s<>		= 24 H	HOUR	REA	DIN	10
							Α	UTO HAI	ME	R		PAG	E 3	OI	=

LOGG	ED BY: I	Blake \$	Summers	1	ATION:	1062.0) ft								
DATE	DRILLE	D: Apri	il 9, 2014	BORII	NG DE	PTH: 70) ft								
DRILL	ING ME	THOD:	Hollow Stem Auger		WATE		EL: 48.	0 ft				R LEVE	EL: 48	.0 ft	
				LAB	RESU	LTS			FII	ELD	DA	TA			
DEPTH (feet)	ELEVATION (feet)	GRAPHIC LOG	SAMPLE DESCRIPTION	NATURAL MOISTURE CONTENT (%)	LIQUID LIMIT (%)	PLASTIC INDEX (%)	BLOW COUNTS	1	N	l Va		, blows/ 0	′ft.	50	100
			FILL: Red-brown, sandy SILT (ML), trace rock fragments												
-	1059.0		Red-brown, silty medium to fine SAND (SM), trace				4 5 7								
5.0	1056.0		rock fragments				7 13 13								\prod
-	1054.0		Red-brown, silty medium to fine SAND (SM), and rock fragments Red-brown, silty medium to fine SAND (SM), some				10 21 6								
10.0_	1052.0		rock fragments Red-brown, sandy SILT (ML), trace rock fragments				5 5 5							\parallel	
15.0							356					•			
20.0	1042.0		Red-brown, sandy SILT (ML), trace rock				4 7 5					•			
- 25.0 _	1037.0		fragments, trace organic material				5 7 7					•			
	-		Red-brown, sandy SILT (ML), some rock fragments				4 6 7					•			
25.0	1032.0	***	RESIDUUM: Medium dense, tan-brown, silty fine SAND (SM)				7 6 9 8					•			
35.0_	1027.0		Firm, tan-red-brown, sandy SILT (ML) Continued Next Page				8							+	\prod
	CO	NT(DUR ENGINEERING, LLC	Ā	= TIME	OF BO	ORING	LEGE (TOB) REMA		Ţ	= 2	24 HOU	IR RE	ADI	۱G
							A	JTO HA	AMME	ER		PA	AGE	1 0	F ;

		e Summers	ELEV	ATION	: 1062.0								_
		oril 9, 2014 D: Hollow Stem Auger			PTH: 70		0.61	20.14	/A T.		/E1 - 4/		_
DRILLING	METHO	D. Hollow Stern Auger		RESU	R LEV	EL: 48.		FIEL		ER LE\	/EL: 48	ι.υ π	_
DEPTH (feet)	GRAPHIC LOG		NATURAL MOISTURE CONTENT (%)	LIQUID LIMIT (%)	PLASTIC INDEX (%)	BLOW COUNTS	1		/alue	e, blow:	s/ft.	50	10
40.0 102	22.0	Firm, tan-red-brown, sandy SILT (ML) (Continued)				4 4 4							
45.0 101		Loose, tan-brown-black, silty medium to fine SAND (SM), moist				455				•			
		Loose, gray-tan, silty fine SAND (SM), wet				345							
50.0 _ 101	07.0	Medium dense, gray-tan, silty fine SAND (SM), moist	_			5 4 6 10				•			
60.0		Dense, gray-tan, silty coarse to fine SAND (SM), moist				17 20 16							
65.0 _ 99	7.0					15 18 20						•	
		Very dense, tan-gray-green, silty fine SAND (SM)				27 27 27 27							
70.0993	<u></u> 0	Boring Terminated at 70 feet.				21							
C	CONT	OUR ENGINEERING, LLC	Ā	= TIME	OF BO		REMARK	S.		24 HO	UR RE	ADI	NC
						Al	JTO HAM	IVIEK	•	F	PAGE	2 0	F

LOGGI	ED BY:	Andrev	v Rebeiz	ELEV	ATION:	: 1063.3	3 ft								
DATE	DRILLE	D: Feb	ruary 10, 2012	BORI	NG DE	PTH: 20	O ft								
DRILLI	DRILLING METHOD: Hollow Stem Auger					R LEV	EL: ft					ER LEV	EL: ft		
				LAE	RESU	LTS			F	IEL	D D	DATA			
DEPTH (feet)	ELEVATION (feet)	GRAPHIC LOG	SAMPLE DESCRIPTION	NATURAL MOISTURE CONTENT (%)				1		N V 5		e, blows	s/ft.	50	10
			FILL: Orange-tan silty SAND (SM), with rock fragments,				_								П
- - -	1060.3		Red-brown sandy SILT (ML), with rock fragments				5 6 8 5 7 14								
5.0 _	<u> </u>						14			+	+				+
-							4 5 5								
10.0 _							5 7 8								
10.0 _							0								
15.0 _							5 6 8								#
	1046.3		RESIDUUM: Medium dense, tan-brown silty SAND (SM)				5 7 10								
-			Boring Terminated at 20 feet.				10								
25.0															
35.0_											Ш				\parallel
_															
	CO	NTO	DUR ENGINEERING, LLC	Σ	= TIME	OF BO	DRING				<u>▼</u> =	: 24 HO	UR RE	ADI	INC
			,				AUTO	REMA DMATI			<u>/IER</u>		AGE	1 0	

LOGG	LOGGED BY: Andrew Rebeiz					ELEVATION: 1064.5 ft													
				BORING DEPTH: 20 ft 24 HR WATER LEVEL: ft TOB WATER LEVEL															
DRILLI	ING ME	D: February 10, 2012 THOD: Hollow Stem Auger SAMPLE DESCRIPTION FILL: Red-brown sandy SILT (ML), with roc fragments, with organic material consistin topsoil with wood fragments and rock fragments and rock fragments and sandy SILT (ML), with rock fragments and sandy SILT (ML), with organic material consisting topsoil with wood fragments and rock fragme	Hollow Stem Auger				EL: ft					R LEVE	L: ft	ft					
				LAB	RESU	LTS			FII	ELD	DA	ATA							
DEPTH (feet)	ELEVATION (feet)	GRAPHIC LOG	SAMPLE DESCRIPTION	NATURAL MOISTURE CONTENT (%)	LIQUID LIMIT (%)	PLASTIC INDEX (%)	BLOW COUNTS	1	Ν	I Va 5		, blows/f		50 <i>-</i>	100				
-	1061.5	1/ 1/	FILL: Red-brown sandy SILT (ML), with rock fragments, with organic material consisting of topsoil with wood fragments and rock fragments				335				•								
5.0_	_		Red-brown sandy SILT (ML), with rock fragments				4 5 8												
-							6 8 12												
10.0							5 7 7					 		\parallel					
- - - 15.0 _							545				4								
-	1047.5		material consisting of topsoil with wood fragment	s			4 6 8												
20.0_	1044.5		Boring Terminated at 20 feet.				. š							+	Н				
25.0														+					
30.0																			
35.0_	-																		
	CO	NT(DUR ENGINEERING, LLC	Ā	= TIME	OF BO	ORING	(TOB) REMA		Ā	= 2	24 HOUF	R REA	<u>IID</u>	١G				
							AUTO	DMATIC		ИМЕ	<u>R</u>	PA	GE 1	Oi	= 1				

LOGGED BY: And	ELEVATION: 1066.6 ft											
DATE DRILLED: F	•	BORII	NG DEI	PTH: 20	O ft							
DRILLING METHO	DD: Hollow Stem Auger		WATE		EL: ft				ER LE\	/EL: f	t	
DEPTH (feet) ELEVATION (feet)	SAMPLE DESCRIPTION	NATURAL MOISTURE CONTENT (%)	LIQUID LIMIT (%)	PLASTIC INDEX (%) 기	BLOW COUNTS				OATA e, blow	s/ft.		
1063.6	fragments, with organic material consisting of	Ž		<u>а</u>	456 455	1		5	10		50	100
5.0 _					5 7 10 5 6 8					,		
15.0					4 4 4							
20.0 1046.6					4 6 10							
	Boring Terminated at 20 feet.											
30.0												
25.0												
CON	TOUR ENGINEERING, LLC	⊻	= TIME	OF BO		LEGE (TOB) REMA	RKS			UR RI		

LOGG	ED BY:	Andrev	v Rebeiz	ELEV		: 1064.	1 ft									_				
DATE	DRILLE	D: Feb	ruary 10, 2012	BORII	NG DE	PTH: 4	O ft													
DRILL	ING ME	THOD:	Hollow Stem Auger	24 HR WATER LEVEL: ft TOB WATER LEVEL: ft																
				LAB	RESU	LTS			FI	ELD	D.	ATA								
DEPTH (feet)	ELEVATION (feet)	GRAPHIC LOG	SAMPLE DESCRIPTION	NATURAL MOISTURE CONTENT (%)	NATURAL MOISTURE CONTENT (%) LIQUID LIMIT (%) PLASTIC INDEX (%)			1	١	N Va		e, blow	rs/ft.	50 1						
			FILL: Red-brown sandy SILT (ML), with rock													П				
5.0 _ - - - -	1058.1		Red-brown sandy SILT (ML), with rock fragments, with organic material consisting of topsoil with wood fragments and rock fragments Red-brown sandy SILT (ML), with rock fragments				345 455 468									_				
40.0		\bowtie	red brown salidy off (ME), with rock fragments				5 7 9					[
10.0 _							5 7 7					•								
-	40474	\bowtie										/								
20.0	1047.1		Red-brown-tan sandy SILT (ML), with rock fragments				4 5 5													
25.0	1037.1		Tan-brown silty SAND (SM), with rock fragments, moist				3 4 5									_				
30.0	-		Red-brown sandy SILT (ML), with rock fragments, moist	-			4 5 7					•								
25.0 _ - - - - - - - - - - - - - - - - - - -	1032.1		Red-brown sandy SILT (ML), organic material consisting of topsoil with wood fragments and rock fragments moist Continued Next Page				5 6 6					•				 				
	•		Continued Hourt age					LEGE	ND							ш				
	CO	NT(DUR ENGINEERING, LLC	Ā	= TIME	OF BO		(TOB) REMA	RKS			24 HC	UR R	EAD	OIN	G				
							AUTO	OMATIO	HAI ز	MM	<u>E</u> R		PAGE		~-	_				